File: 16158

PINKHAM & GREER CIVIL ENGINEERS

> 28 VANNAH AVE. PORTLAND, ME. 04103 Tel: 207.781.5242 Fax: 207.781.4245

April 12, 2017

Ms. Carla Nixon Town of Cumberland 290 Tuttle Rd Cumberland, ME 04021

RE: SOLAR WAY SUBDIVISION, RESPONSE TO STAFF COMMENTS

Dear Carla,

Enclosed are revised drawings addressing Dan Diffin's comments. Below is a response to the Conservation Commission questions.

- Attached is the latest Stormwater Model.
- 2. The net residential calculations are the same on both plans.
- 3. The setbacks on each lot have been adjusted.
- 4. The turnaround dimensions have been revised.

Please note we have revised the culvert crossings to provide a natural stream bed for better passage of wildlife.

Conservation Commission

1. There are wetland designations for both Lots 4 and 5 in close proximity to the housing envelopes. Are these wetlands being protected or filled in during or after construction?

No filling of the wetlands beyond the stream crossing will occur.

2. Given that the drainage is to the Piscataqua watershed, there could be short or long-term impacts to the stream water quality. And if the road is paved in the future, which is allowed, then the run-off would aggravate the potential contamination. Are any special measures other than the 4" loam and seed being required to retain potential contaminants from reaching the stream even though the planned flow leaving the property is less then that generated on the property?

No additional stormwater treatments or control are planned for the project. Ms. Carla Nixon April 12, 2017 Page 2 of 3

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3. The homeowners association by definition consists of a maximum five interested or invested parties responsible for maintaining and enforcing the final plan elements including maintenance and operation of the association. Failure to adhere to the approved final plan could impact the environmental conditions in the subdivision as well as downstream. The Homeowners Association meeting results should be provided on an annual basis to the Codes Office for compliance to that final plan.

Currently the homeowners will be required to maintain the road and their own properties. No additional reporting is required, unless the MS4 Regulations require it.

4. Will the existing yurt be permitted? If so, there are no water, waste treatment or power requirements designated in the final plan.

The yurt is currently permitted. There are no plans to provide sewer and water service to the yurt.

5. In section 4, the DEP application for driveway construction, the PBR list, Section 6, Movement of Rocks or Vegetation is not checked. Was this an oversight?

The Stream Crossing Section covers the installation of the culverts as shown on the plans. Movement of rocks or vegetation is a separate section for work not including a stream crossing.

6. Section 7, Inspection and Maintenance of Stormwater Management Facilities: Are the inspection reports done on a scheduled basis and filed in the Town offices for review?

The Homeowners Association will file reports with the Town if required under the MS4 Regulaitons.

7. Who are the inspectors? And who has responsibility for clean-up activities? Finally, is the town responsible for maintaining the "Housekeeping Report" including inspection and maintenance?

The Homeowners Association and the Homeowners are required to comply with the housekeeping requirements and maintaining the road, ditches and culverts in a stable condition.

Please note we have attached the current DEP NRPA Permit.

Ms. Carla Nixon April 12, 2017 Page 3 of 3 File: 16158



Please let me know if you have any additional comments.

Sincerely,

PINKHAM & GREER, CIVIL ENGINEERS

Thomas S. Greer, P.E.

cc: Patrice Miller, Dan Diffin, File

Enclosures TSG/rjs



STORMWATER MANAGEMENT REPORT SOLAR WAY SUBDIVISION CUMBERLAND, MAINE

28 VANNAH AVE. PORTLAND, ME. 04103 Tel: 207.781.5242 Fax: 207.781.4245

November 28, 2016 Revised February 6, 2017 Revised April 12, 2017

Project Description:

Solar Way Subdivision will provide housing for 3 additional homes. Currently there are two existing homes on the 24.84 acre property. This project creates a five lot clustered subdivision. The site is located on the southerly side of Hillside Avenue near the Yarmouth-Cumberland town line.

The project will have 750 feet of private road servicing the 5 homes. The power and communication systems will be underground.

Existing Conditions:

The existing site has two fully developed homes. The front of the site is fairly open. It was clear cut not long ago. The rear of the site has some additional forested area. There are tote roads that access the full site.

There are wetlands that transect the site. These are stream based with defined channels, and easily seen on the existing conditions plan sheet C1.0.

The soils on site are buxton, scantic, and limerick-saco based on the Cumberland County Soils Medium Intensity Soils Map. These soils are heavily clay based soils.

Developed Conditions:

The developed conditions converts the existing driveway into a private gravel road 18' wide. The front 4 lots will be approximately 60,000 sq. ft. The last lot will be 7.8 acres. The rear of the parcel will be left undeveloped open space of 10.2 acres.

Drainage:

The existing drainage flows generally from east to west. The drainage basin is approximately 64 acres. The area above the site is partially open space with the power lines and a residential subdivision in the Yarmouth section.

Methodology:

This stormwater analysis was performed using HydroCad Software based on TR-55 modeling conditions. This model requires assumptions as to the land cover, slopes and soils. These are enhanced by the topography mapping, soils mapping, and on-site observations. The flows were determined using a Type III coastal storm and rainfall totals for the 24 hour period for a 1 year storm, 2.6", for a 2 year storm, 3.1", for a 10 year storm, 4.6", and for a 25 year storm, 5.8". These data are published in the manual for Stormwater Management for Maine: Best Management Practices, published by the Maine Department of Environmental Protection.

Model:

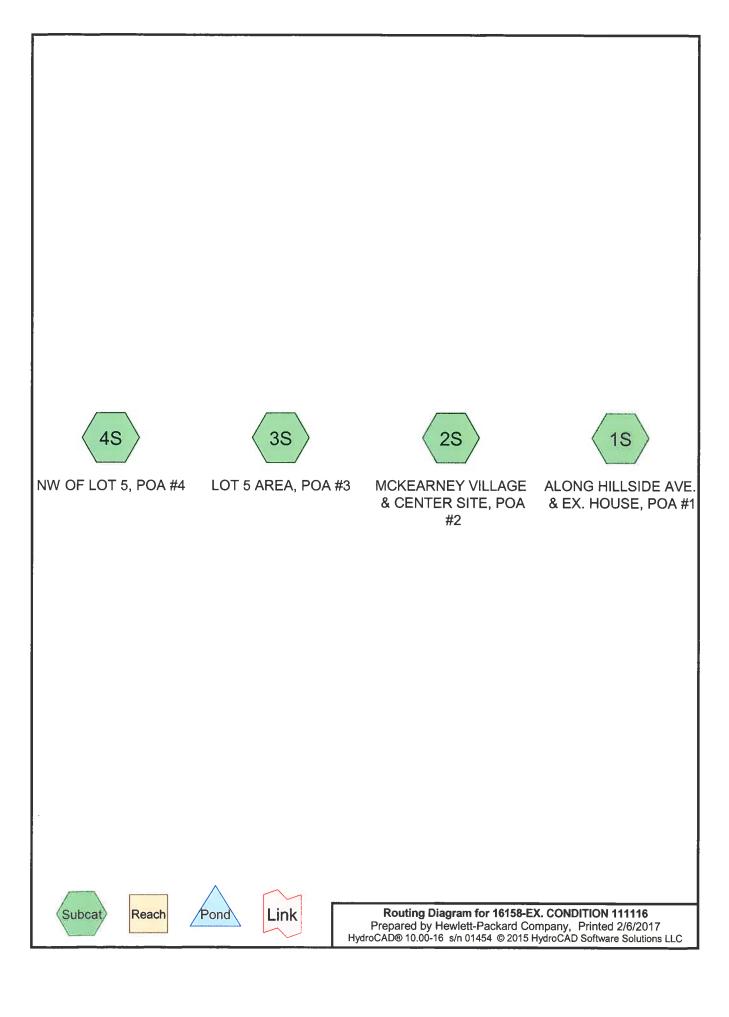
The hydrocad model was developed using 4 points of analysis corresponding to the swales and streams that cross the site. The peak flows leaving the site are equal to or less than the existing conditions flow. The two steam crossings create a small delay in the peak flows.

	TABLE 1							
			PEAK	FLOWS (CFS)			
		Exi	sting		Developed			
POA	2 yr	10 yr	25 yr	100 yr	2 yr	10 yr	25 yr	100 yr
1	2.07	4.54	6.66	11.02	1.82	3.84	5.54	8.98
2	19.55	42.23	61.57	101.33	14.32	39.08	58.16	95.97
3	0.60	1.69	2.70	4.89	0.60	1.69	2.70	4.89
4	2.84	7.47	11.71	20.87	2.84	7.47	11.71	20.87

Conclusions:

This project makes a very light impact on the 24 acre site, with 3 additional homes. There will be a net reduction in peak flows leaving the site. There should be no unreasonable impact on downstream properties as a result of stormwater flows from this site.

No. 4200



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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: ALONG HILLSIDE AVE. Runoff Area=165,284 sf 1.39% Impervious Runoff Depth>0.90" Flow Length=878' Tc=39.5 min CN=74 Runoff=2.07 cfs 0.283 af

Subcatchment 2S: MCKEARNEY

Runoff Area=2,058,116 sf 3.54% Impervious Runoff Depth>0.93"
Flow Length=2,764' Tc=70.4 min CN=75 Runoff=19.55 cfs 3.676 af

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>0.54" Flow Length=652' Tc=37.1 min CN=66 Runoff=0.60 cfs 0.089 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>0.60" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=2.84 cfs 0.686 af

Total Runoff Area = 66.833 ac Runoff Volume = 4.734 af Average Runoff Depth = 0.85" 97.42% Pervious = 65.109 ac 2.58% Impervious = 1.724 ac Prepared by Hewlett-Packard Company

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: ALONG HILLSIDE AVE. Runoff Area=165,284 sf 1.39% Impervious Runoff Depth>1.91" Flow Length=878' Tc=39.5 min CN=74 Runoff=4.54 cfs 0.603 af

Subcatchment 2S: MCKEARNEY

Runoff Area=2,058,116 sf 3.54% Impervious Runoff Depth>1.96"

Flow Length=2,764' Tc=70.4 min CN=75 Runoff=42.23 cfs 7.707 af

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>1.34" Flow Length=652' Tc=37.1 min CN=66 Runoff=1.69 cfs 0.224 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>1.44" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=7.47 cfs 1.651 af

Total Runoff Area = 66.833 ac Runoff Volume = 10.184 af Average Runoff Depth = 1.83" 97.42% Pervious = 65.109 ac 2.58% Impervious = 1.724 ac

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: ALONG HILLSIDE AVE. Runoff Area=165,284 sf 1.39% Impervious Runoff Depth>2.79" Flow Length=878' Tc=39.5 min CN=74 Runoff=6.66 cfs 0.882 af

Subcatchment 2S: MCKEARNEY

Runoff Area=2,058,116 sf 3.54% Impervious Runoff Depth>2.85"

Flow Length=2,764' Tc=70.4 min CN=75 Runoff=61.57 cfs 11.207 af

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>2.10" Flow Length=652' Tc=37.1 min CN=66 Runoff=2.70 cfs 0.349 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>2.21" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=11.71 cfs 2.537 af

Total Runoff Area = 66.833 ac Runoff Volume = 14.976 af Average Runoff Depth = 2.69" 97.42% Pervious = 65.109 ac 2.58% Impervious = 1.724 ac

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Type III 24-hr 100-YEAR Rainfall=8.10"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: ALONG HILLSIDE AVE. Runoff Area=165,284 sf 1.39% Impervious Runoff Depth>4.65" Flow Length=878' Tc=39.5 min CN=74 Runoff=11.02 cfs 1.469 af

Subcatchment 2S: MCKEARNEY Runoff Area=2,058,116 sf 3.54% Impervious Runoff Depth>4.71" Flow Length=2,764' Tc=70.4 min CN=75 Runoff=101.33 cfs 18.545 af

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>3.76" Flow Length=652' Tc=37.1 min CN=66 Runoff=4.89 cfs 0.626 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>3.89" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=20.87 cfs 4.473 af

> Total Runoff Area = 66.833 ac Runoff Volume = 25.113 af Average Runoff Depth = 4.51" 97.42% Pervious = 65.109 ac 2.58% Impervious = 1.724 ac

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Page 1

Summary for Subcatchment 1S: ALONG HILLSIDE AVE. & EX. HOUSE, POA #1

Runoff

6.66 cfs @ 12.55 hrs, Volume=

0.882 af, Depth> 2.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YEAR Rainfall=5.82"

	Α	rea (sf)	CN E	Description		
	27,632 70 Woods, Good, HSG C					
		5,743	77 V	Voods, Go	od, HSG D	
		15,040	73 E	Brush, Goo	d, HSG D	
		38,556	65 E	Brush, Goo	d, HSG C	
		47,688	74 >	75% Gras	s cover, Go	ood, HSG C
		16,290	80 >	75% Gras	s cover, Go	ood, HSG D
		2,303	98 F	Roofs, HSG	C	
*		12,032	96 (Gravel surfa	ace,	
_	1	65,284	74 V	Veighted A	verage	
	1	62,981	9	8.61% Per	vious Area	
		2,303	1	.39% Impe	ervious Area	a
				•		
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	28.6	100	0.0400	0.06		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.14"
	3.6	248	0.0520	1.14		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	7.3	530	0.0300	1.21		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	39.5	878	Total			

Summary for Subcatchment 2S: MCKEARNEY VILLAGE & CENTER SITE, POA #2

Runoff

61.57 cfs @ 12.94 hrs, Volume=

11.207 af, Depth> 2.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YEAR Rainfall=5.82"

	Area (sf)	CN	Description
-	783,682	77	Woods, Good, HSG D
	274,538	70	Woods, Good, HSG C
*	249,967	74	Woods, Good, HSG C/D
	455,273	73	Brush, Good, HSG D
	212,756	74	>75% Grass cover, Good, HSG C
	18,878	98	Roofs, HSG C
	9,099	96	Gravel surface, HSG C
	53,923	98	Paved parking, HSG C
	2,058,116	75	Weighted Average
	1,985,315		96.46% Pervious Area
	72,801		3.54% Impervious Area

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	44.0	130	0.0230	0.05	(5.5)	Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.14"
	22.9	688	0.0100	0.50		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.1	519	0.0150	8.16	359.12	
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
						n= 0.040 Mountain streams
	0.4	294	0.0410	13.49	593.73	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
						n= 0.040 Mountain streams
	1.4	759	0.0180	8.94	393.40	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
		004	0.0450	0.40	050.40	n= 0.040 Mountain streams
	0.5	264	0.0150	8.16	359.12	•
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
	0.4	440	0.0550	45.00	007.07	n= 0.040 Mountain streams
	0.1	110	0.0550	15.63	687.67	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00' n= 0.040 Mountain streams
_	=0.4	0.704				11- U.U4U IVIUUIIIAIII SIICAIIIS
	70.4	2,764	Total			

Summary for Subcatchment 3S: LOT 5 AREA, POA #3

Runoff = 2.70 cfs @ 12.54 hrs, Volume=

0.349 af, Depth> 2.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YEAR Rainfall=5.82"

	A	rea (sf)	CN	Description		2001.
		79,236	65	Brush, Goo	d, HSG C	
		1,266	96	Gravel surfa	ace, HSG C	
		6,629	70	Woods, Go	od, HSG C	
		87,131	66	Weighted A	verage	
		87,131		100.00% Pe	ervious Are	a
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	
	30.0	130	0.0150	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.14"
	5.9	415	0.0280	1.17		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	1.2	107	0.0900	1.50		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
_	37.1	652	Total			

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Summary for Subcatchment 4S: NW OF LOT 5, POA #4

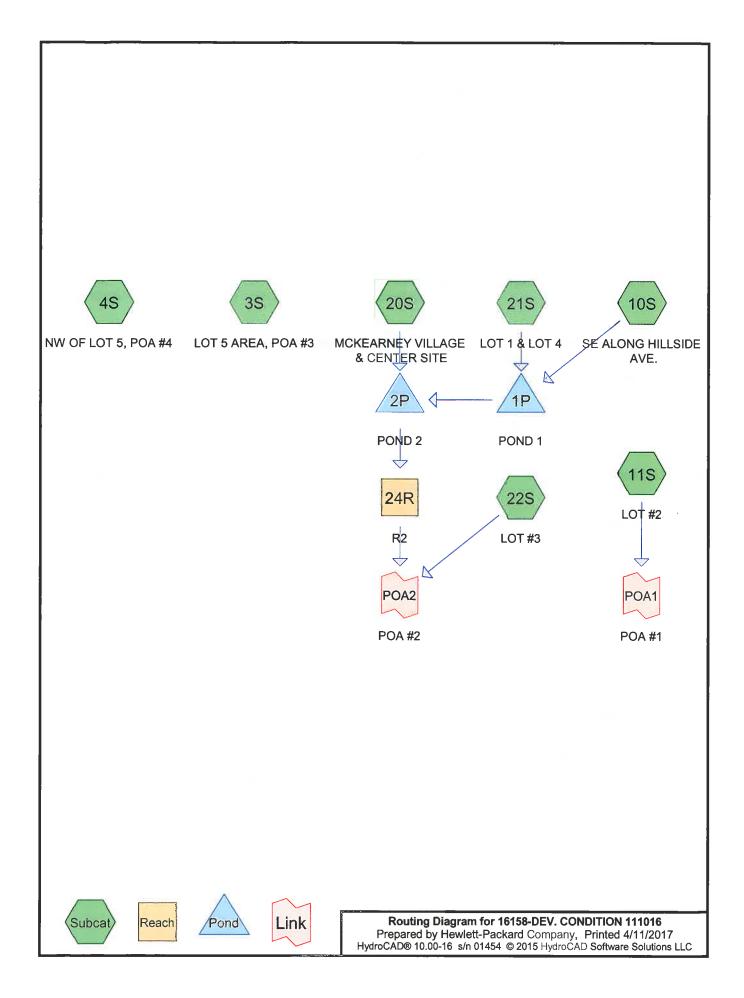
Runoff = 11.71 cfs @

11.71 cfs @ 13.25 hrs, Volume=

2.537 af, Depth> 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25-YEAR Rainfall=5.82"

	Α	rea (sf)	CN E	Descri ption		
		64,570	73 E	Brush, Goo	d, HSG D	
		94,451	77 V	Voods, Go	od, HSG D	
		49,002			od, HSG C	
		01,853		Woods, Good, HSG B		
		77,820		Brush, Goo	,	
		11,812			,	ood, HSG D
_		1,221			ace, HSG D	
		00,729		Veighted A	-	
	6	00,729	1	00.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	56.1	110	0.0090	0.03		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.14"
	32.1	1,397	0.0210	0.72		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	3.9	524	0.0340	2.26	22.63	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=1.00' Z= 3.0 '/' Top.W=13.00'
_						n= 0.100 Heavy timber, flow below branches
	92.1	2,031	Total			



Type III 24-hr 2-YEAR Rainfall=3.14"

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>0.54" Flow Length=652' Tc=37.1 min CN=66 Runoff=0.60 cfs 0.089 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>0.60" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=2.84 cfs 0.686 af

Subcatchment 10S: SE ALONG HILLSIDE Runoff Area=67,013 sf 5.93% Impervious Runoff Depth>1.12" Flow Length=638' Tc=36.2 min CN=78 Runoff=1.12 cfs 0.143 af

Subcatchment 11S: LOT #2 Runoff Area=98,914 sf 2.38% Impervious Runoff Depth>1.01" Flow Length=322' Tc=22.1 min CN=76 Runoff=1.82 cfs 0.191 af

Subcatchment 20S: MCKEARNEY

Runoff Area=1,557,537 sf 4.49% Impervious Runoff Depth>0.93"
Flow Length=2,478' Tc=70.4 min CN=75 Runoff=14.79 cfs 2.782 af

Subcatchment 21S: LOT 1 & LOT 4 Runoff Area=426,493 sf 1.87% Impervious Runoff Depth>0.93" Flow Length=1,073' Tc=79.1 min CN=75 Runoff=3.78 cfs 0.758 af

Subcatchment 22S: LOT #3 Runoff Area=73,454 sf 2.73% Impervious Runoff Depth>1.01" Flow Length=177' Tc=15.1 min CN=76 Runoff=1.58 cfs 0.142 af

Reach 24R: R2 Avg. Flow Depth=0.49' Max Vel=2.49 fps Inflow=14.15 cfs 2.668 af

n=0.070 L=237.0' S=0.0422 '/' Capacity=390.24 cfs Outflow=14.11 cfs 2.657 af

Pond 1P: POND 1 Peak Elev=97.20' Storage=5,482 cf Inflow=4.26 cfs 0.901 af

Outflow=4.25 cfs 0.790 af

Pond 2P: POND 2 Peak Elev=95.14' Storage=48,926 cf Inflow=19.02 cfs 3.572 af

Outflow=14.15 cfs 2.668 af

Link POA1: POA #1 Inflow=1.82 cfs 0.191 af

Primary=1.82 cfs 0.191 af

Link POA2: POA #2 Inflow=14.32 cfs 2.799 af

Primary=14.32 cfs 2.799 af

Total Runoff Area = 66.834 ac Runoff Volume = 4.792 af Average Runoff Depth = 0.86" 97.04% Pervious = 64.853 ac 2.96% Impervious = 1.980 ac HydroCAD® 10.00-16 s/n 01454 © 2015 HydroCAD Software Solutions LLC

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>1.34" Flow Length=652' Tc=37.1 min CN=66 Runoff=1.69 cfs 0.224 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>1.44" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=7.47 cfs 1.651 af

Subcatchment 10S: SE ALONG HILLSIDE Runoff Area=67,013 sf 5.93% Impervious Runoff Depth>2.22" Flow Length=638' Tc=36.2 min CN=78 Runoff=2.25 cfs 0.285 af

Subcatchment 11S: LOT #2 Runoff Area=98,914 sf 2.38% Impervious Runoff Depth>2.07" Flow Length=322' Tc=22.1 min CN=76 Runoff=3.84 cfs 0.393 af

Subcatchment 20S: MCKEARNEY

Runoff Area=1,557,537 sf 4.49% Impervious Runoff Depth>1.96"

Flow Length=2,478' Tc=70.4 min CN=75 Runoff=31.96 cfs 5.832 af

Subcatchment 21S: LOT 1 & LOT 4

Runoff Area=426,493 sf 1.87% Impervious Runoff Depth>1.95"

Flow Length=1,073' Tc=79.1 min CN=75 Runoff=8.17 cfs 1.591 af

Subcatchment 22S: LOT #3

Runoff Area=73,454 sf 2.73% Impervious Runoff Depth>2.08"

Flow Length=177' Tc=15.1 min CN=76 Runoff=3,33 cfs 0.292 af

Reach 24R: R2 Avg. Flow Depth=0.88' Max Vel=3.48 fps Inflow=38.69 cfs 6.662 af

n=0.070 L=237.0' S=0.0422 '/' Capacity=390.24 cfs Outflow=38.64 cfs 6.647 af

Pond 1P: POND 1 Peak Elev=97.32' Storage=6,097 cf Inflow=9.13 cfs 1.876 af

Outflow=9.12 cfs 1.764 af

Pond 2P: POND 2 Peak Elev=95.78' Storage=63,039 cf Inflow=41.02 cfs 7.596 af

Outflow=38.69 cfs 6.662 af

Link POA1: POA #1 Inflow=3.84 cfs 0.393 af

Primary=3.84 cfs 0.393 af

Link POA2: POA #2 Inflow=39.08 cfs 6.939 af

Primary=39.08 cfs 6.939 af

Total Runoff Area = 66.834 ac Runoff Volume = 10.268 af Average Runoff Depth = 1.84" 97.04% Pervious = 64.853 ac 2.96% Impervious = 1.980 ac

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>2.10"

Flow Length=652' Tc=37.1 min CN=66 Runoff=2.70 cfs 0.349 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>2.21" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=11.71 cfs 2.537 af

Subcatchment 10S: SE ALONG HILLSIDE Runoff Area=67,013 sf 5.93% Impervious Runoff Depth>3.17"

Flow Length=638' Tc=36.2 min CN=78 Runoff=3.19 cfs 0.406 af

Subcatchment 11S: LOT #2 Runoff Area=98,914 sf 2.38% Impervious Runoff Depth>2.99"

Flow Length=322' Tc=22.1 min CN=76 Runoff=5.54 cfs 0.566 af

Subcatchment 20S: MCKEARNEY Runoff Area=1,557,537 sf 4.49% Impervious Runoff Depth>2.85"

Flow Length=2,478' Tc=70.4 min CN=75 Runoff=46.59 cfs 8.481 af

Subcatchment 21S: LOT 1 & LOT 4 Runoff Area=426,493 sf 1.87% Impervious Runoff Depth>2.84"

Flow Length=1,073' Tc=79.1 min CN=75 Runoff=11.90 cfs 2.314 af

Subcatchment 22S: LOT #3 Runoff Area=73,454 sf 2.73% Impervious Runoff Depth>3.00"

Flow Length=177' Tc=15.1 min CN=76 Runoff=4.79 cfs 0.421 af

Reach 24R: R2 Avg. Flow Depth=1.10' Max Vel=3.95 fps Inflow=57.59 cfs 10.132 af

n=0.070 L=237.0' S=0.0422'/' Capacity=390.24 cfs Outflow=57.52 cfs 10.115 af

Pond 1P: POND 1 Peak Elev=97.41' Storage=6,576 cf Inflow=13.27 cfs 2.720 af

Outflow=13.26 cfs 2.607 af

Pond 2P: POND 2 Peak Elev=96.17' Storage=72,926 cf Inflow=59.78 cfs 11.088 af

Outflow=57.59 cfs 10.132 af

Link POA1: POA #1 Inflow=5.54 cfs 0.566 af

Primary=5.54 cfs 0.566 af

Link POA2: POA #2 Inflow=58.16 cfs 10.536 af

Primary=58.16 cfs 10.536 af

Total Runoff Area = 66.834 ac Runoff Volume = 15.076 af Average Runoff Depth = 2.71" 97.04% Pervious = 64.853 ac 2.96% Impervious = 1.980 ac

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 3S: LOT 5 AREA, POA #3 Runoff Area=87,131 sf 0.00% Impervious Runoff Depth>3.76" Flow Length=652' Tc=37.1 min CN=66 Runoff=4.89 cfs 0.626 af

Subcatchment 4S: NW OF LOT 5, POA #4 Runoff Area=600,729 sf 0.00% Impervious Runoff Depth>3.89" Flow Length=2,031' Tc=92.1 min CN=68 Runoff=20.87 cfs 4.473 af

Subcatchment 10S: SE ALONG HILLSIDE Runoff Area=67,013 sf 5.93% Impervious Runoff Depth>5.11" Flow Length=638' Tc=36.2 min CN=78 Runoff=5.08 cfs 0.655 af

Subcatchment 11S: LOT #2

Runoff Area=98,914 sf 2.38% Impervious Runoff Depth>4.90"
Flow Length=322' Tc=22.1 min CN=76 Runoff=8.98 cfs 0.928 af

Subcatchment 20S: MCKEARNEY

Runoff Area=1,557,537 sf 4.49% Impervious Runoff Depth>4.71"

Flow Length=2,478' Tc=70.4 min CN=75 Runoff=76.68 cfs 14.034 af

Subcatchment 21S: LOT 1 & LOT 4 Runoff Area=426,493 sf 1.87% Impervious Runoff Depth>4.69" Flow Length=1,073' Tc=79.1 min CN=75 Runoff=19.52 cfs 3.831 af

Subcatchment 22S: LOT #3

Runoff Area=73,454 sf 2.73% Impervious Runoff Depth>4.91"
Flow Length=177' Tc=15.1 min CN=76 Runoff=7.77 cfs 0.690 af

Reach 24R: R2 Avg. Flow Depth=1.44' Max Vel=4.59 fps Inflow=95.05 cfs 17.416 af n=0.070 L=237.0' S=0.0422 '/' Capacity=390.24 cfs Outflow=94.96 cfs 17.393 af

Pond 1P: POND 1 Peak Elev=97.55' Storage=7,472 cf Inflow=21.75 cfs 4.486 af Outflow=21.73 cfs 4.370 af

Pond 2P: POND 2 Peak Elev=96.83' Storage=91,748 cf Inflow=98.20 cfs 18.405 af

Outflow=95.05 cfs 17.416 af

Link POA1: POA #1 Inflow=8.98 cfs 0.928 af
Primary=8.98 cfs 0.928 af

Link POA2: POA #2 Inflow=95.97 cfs 18.084 af Primary=95.97 cfs 18.084 af

Total Runoff Area = 66.834 ac Runoff Volume = 25.236 af Average Runoff Depth = 4.53" 97.04% Pervious = 64.853 ac 2.96% Impervious = 1.980 ac

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Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
7.775	75	>75% Grass cover, Good, HSG C (10S, 11S, 20S, 21S, 22S)
0.271	80	>75% Grass cover, Good, HSG D (4S)
2.740	75	>75% Grass cover, Good, HSG D (10S, 11S, 20S, 21S, 22S)
3.784	65	Brush, Good, HSG C (3S, 4S, 21S)
8.338	73	Brush, Good, HSG D (4S, 20S, 21S)
0.594	96	Gravel surface, HSG C (3S, 10S, 11S, 20S, 21S, 22S)
0.028	96	Gravel surface, HSG D (4S)
1.329	98	Paved parking, HSG C (10S, 20S)
0.101	98	Paved roads w/curbs & sewers, HSG C (21S)
0.550	98	Roofs, HSG C (11S, 20S, 21S, 22S)
3.004	55	Woods, Good, HSG B (4S, 20S)
12.895	70	Woods, Good, HSG C (3S, 4S, 10S, 11S, 20S, 21S, 22S)
5.504	74	Woods, Good, HSG C/D (20S, 21S)
19.919	77	Woods, Good, HSG D (4S, 11S, 20S, 21S, 22S)

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Summary for Subcatchment 3S: LOT 5 AREA, POA #3

Runoff

0.60 cfs @ 12.62 hrs, Volume=

0.089 af, Depth> 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

A	rea (sf)	CN E	Descri ption		
	79,236	65 E	Brush, Goo	d, HSG C	
	1,266	96 (Gravel surfa	ace, HSG C	
	6,629	70 V	Voods, Go	od, HSG C	
	87,131	66 V	Veighted A	verage	
	87,131	1	100.00% Pe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
30.0	130	0.0150	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.14"
5.9	415	0.0280	1.17		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
1.2	107	0.0900	1.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
37.1	652	Total			

Summary for Subcatchment 4S: NW OF LOT 5, POA #4

Runoff

2.84 cfs @ 13.40 hrs, Volume=

0.686 af, Depth> 0.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

Area (sf)	CN	Description
64,570	73	Brush, Good, HSG D
94,451	77	Woods, Good, HSG D
249,002	70	Woods, Good, HSG C
101,853	55	Woods, Good, HSG B
77,820	65	Brush, Good, HSG C
11,812	80	>75% Grass cover, Good, HSG D
1,221 _	96	Gravel surface, HSG D
600,729	68	Weighted Average
600,729		100.00% Pervious Area

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	56.1	110	0.0090	0.03		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.14"
	32.1	1,397	0.0210	0.72		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	3.9	524	0.0340	2.26	22.63	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=1.00' Z= 3.0 '/' Top.W=13.00'
_						n= 0.100 Heavy timber, flow below branches
	92.1	2,031	Total			

Summary for Subcatchment 10S: SE ALONG HILLSIDE AVE.

Runoff = 1.12 cfs @ 12.53 hrs, Volume=

0.143 af, Depth> 1.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

	Δ	rea (sf)	CN	Description						
		14,100	70	Woods, Go	od, HSG C					
		3,976	98	Paved park	ing, HSG C					
*		34,609	75	>75% Gras	s cover, Go	ood, HSG C				
*		7,045	75	>75% Gras	75% Grass cover, Good, HSG D					
_		7 ,283	96	Gravel surfa	ac <mark>e, HSG</mark> (
		67,013	78	Weighted A	verage					
		63,037		94.07% Per	vious Area					
		3,976		5.93% Impe	ervious Are	a				
					_					
	Tc		Slope	•	Capacity	Description				
	(min)	(feet)	(ft/ft		(cfs)					
	28.6	100	0.0400	0.06		Sheet Flow,				
						Woods: Dense underbrush n= 0.800 P2= 3.14"				
	5.7	383	0.0500	1.12		Shallow Concentrated Flow,				
	0.4	0.5	0.040		0.40	Woodland Kv= 5.0 fps				
	0.1	25	0.0100	5.26	6.46	Pipe Channel,				
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'				
	4.0	400	0.0047	4.00		n= 0.013 Corrugated PE, smooth interior				
	1.8	130	0.0310	1.23		Shallow Concentrated Flow,				
_						Short Grass Pasture Kv= 7.0 fps				
	36.2	638	Total							

Summary for Subcatchment 11S: LOT #2

Runoff = 1.82 cfs @ 12.33 hrs, Volume=

0.191 af, Depth> 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

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_	A	rea (sf)	CN [Description					
		5,743	77 \	Woods, Good, HSG D					
		12,538	70 V	Woods, Good, HSG C					
*		50,975	75 >	>75% Grass cover, Good, HSG C					
*		21,996	75 >	>75% Grass cover, Good, HSG D					
		2,358	98 F	Roofs, HSG	C				
_		5,304	96 (Gravel surfa	ace, HSG C				
		98,914	76 \	Veighted A	verage				
		96,556	(7.62% Per	vious Area				
		2,358	2	2.38% Impe	ervious Area	a			
	Тс	Length	Slope		Capacity	Description			
_	_(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	18.4	100	0.0300	0.09		Sheet Flow,			
						Woods: Light underbrush n= 0.400 P2= 3.14"			
	3.7	222	0.0400	1.00		Shallow Concentrated Flow,			
						Woodland Kv= 5.0 fps			
	22.1	322	Total						

Summary for Subcatchment 20S: MCKEARNEY VILLAGE & CENTER SITE

Runoff = 14.79 cfs @ 13.01 hrs, Volume=

2.782 af, Depth> 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

	Area (sf)	CN	Description
	550,006	77	Woods, Good, HSG D
	205,778	70	Woods, Good, HSG C
*	239,400	74	Woods, Good, HSG C/D
	216,390	73	Brush, Good, HSG D
*	213,824	75	>75% Grass cover, Good, HSG C
	16,012	98	Roofs, HSG C
	6,041	96	Gravel surface, HSG C
	53,923	98	Paved parking, HSG C
	29,004	55	Woods, Good, HSG B
*	27,159	75	>75% Grass cover, Good, HSG D
	1,557,537	75	Weighted Average
	1,487,602		95.51% Pervious Area
	69,935		4.49% Impervious Area

Type III 24-hr 2-YEAR Rainfall=3.14"

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	44.0	130	0.0230	0.05		Sheet Flow,
						Woods: Dense underbrush n= 0.800 P2= 3.14"
	22.9	688	0.0100	0.50		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	1.1	519	0.0150	8.16	359.12	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
						n= 0.040 Mountain streams
	0.4	294	0.0410	13.49	593.73	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
						n= 0.040 Mountain streams
	1.8	737	0.0100	6.66	293.22	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
						n= 0.040 Mountain streams
	0.2	110	0.0150	8.16	359.12	Trap/Vee/Rect Channel Flow,
						Bot.W=7.00' D=4.00' Z= 1.0 '/' Top.W=15.00'
_						n= 0.040 Mountain streams
	70.4	2 478	Total			

Summary for Subcatchment 21S: LOT 1 & LOT 4

Runoff = 3.78 cfs @ 13.12 hrs, Volume=

0.758 af, Depth> 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

	Area (sf)	CN	Description
	209,417	77	Woods, Good, HSG D
	58,087	70	Woods, Good, HSG C
*	371	74	Woods, Good, HSG C/D
	82,255	73	Brush, Good, HSG D
*	42,096	75	>75% Grass cover, Good, HSG D
*	16,766	75	>75% Grass cover, Good, HSG C
	3,601	98	Roofs, HSG C
	1,730	96	Gravel surface, HSG C
	4,378	98	Paved roads w/curbs & sewers, HSG C
	7,792	65	Brush, Good, HSG C
	426,493 418,514 7,979	75	Weighted Average 98.13% Pervious Area 1.87% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
49.8	100	0.0100	0.03		Sheet Flow,
4.6	211	0.0230	0.76		Woods: Dense underbrush n= 0.800 P2= 3.14" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
2.6	213	0.0380	1.36		Shallow Concentrated Flow,
16.8	339	0.0180	0.34		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, Forest w/Heavy Litter Kv= 2.5 fps
5.3	210	0.0090	0.66		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
79.1	1,073	Total			

Summary for Subcatchment 22S: LOT #3

Runoff = 1.58 cfs @ 12.22 hrs, Volume=

0.142 af, Depth> 1.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 2-YEAR Rainfall=3.14"

	Α	rea (sf)	CN D	escription					
		8,051	77 V	77 Woods, Good, HSG D					
		15,580	70 V	Woods, Good, HSG C					
*		22,510	75 >	>75% Grass cover, Good, HSG C					
		2,008	98 F	Roofs, HSC	G C				
		4,239	96 G	Fravel surfa	ace, HSG (
*		21,066	75 >	75% Gras	s cover, Go	ood, HSG D			
		73,454	76 V	Veighted A	verage				
		71,446	9	7.27% Per	rvious Area				
		2,008	2	.73% Impe	ervious Are	a			
	Tc	Longth	Clana	Volocity	O	Description			
		Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)	Description	_		
_		•		•		Sheet Flow,			
_	(min)	(feet)	(ft/ft)	(ft/sec)		•			
	(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Grass: Dense n= 0.240 P2= 3.14" Shallow Concentrated Flow,			
	(min) 14.4	(feet) 100	(ft/ft) 0.0200	(ft/sec) 0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.14"	_		
_	(min) 14.4	(feet) 100	(ft/ft) 0.0200	(ft/sec) 0.12		Sheet Flow, Grass: Dense n= 0.240 P2= 3.14" Shallow Concentrated Flow, Woodland Kv= 5.0 fps Shallow Concentrated Flow,	_		
_	(min) 14.4 0.2	(feet) 100 41	(ft/ft) 0.0200 0.3400	(ft/sec) 0.12 2.92		Sheet Flow, Grass: Dense n= 0.240 P2= 3.14" Shallow Concentrated Flow, Woodland Kv= 5.0 fps	_		

Summary for Reach 24R: R2

Inflow Area = 47.085 ac, 3.99% Impervious, Inflow Depth > 0.68" for 2-YEAR event Inflow = 14.15 cfs @ 13.51 hrs, Volume= 2.668 af

Outflow = 14.11 cfs @ 13.56 hrs, Volume= 2.657 af, Atten= 0%, Lag= 2.8 min

Type III 24-hr 2-YEAR Rainfall=3.14"

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Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 1.6 min

Avg. Velocity = 1.23 fps, Avg. Travel Time= 3.2 min

Peak Storage= 1,344 cf @ 13.53 hrs Average Depth at Peak Storage= 0.49'

Bank-Full Depth= 3.00' Flow Area= 57.0 sf, Capacity= 390.24 cfs

10.00' x 3.00' deep channel, n= 0.070 Sluggish weedy reaches w/pools

Side Slope Z-value= 3.0 '/' Top Width= 28.00'

Length= 237.0' Slope= 0.0422 '/'

Inlet Invert= 90.00', Outlet Invert= 80.00'



Summary for Pond 1P: POND 1

Inflow Area = 11.329 ac, 2.42% Impervious, Inflow Depth > 0.95" for 2-YEAR event

Inflow = 4.26 cfs @ 13.08 hrs, Volume= 0.901 af

Outflow = 4.25 cfs @ 13.10 hrs, Volume= 0.790 af, Atten= 0%, Lag= 1.4 min

Primary = 4.25 cfs @ 13.10 hrs, Volume= 0.790 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 97.20' @ 13.10 hrs Surf.Area= 4,439 sf Storage= 5,482 cf

Plug-Flow detention time= 52.9 min calculated for 0.790 af (88% of inflow)

Center-of-Mass det. time= 19.3 min (885.0 - 865.7)

Volume	Inv	ert Avail.Sto	rage Storage	Description	
#1	92.	00' 11,20	68 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
92.0		32	0	0	
94.0	00	485	517 1,955 2,266	517	
96.0	00	1,470		2,472	
97.0		3,061		4,738	
98.0	00	10,000	6,531	11,268	
Device	Routing	Invert	Outlet Device	s	, , , , , , , , , , , , , , , , , , ,
#1	Primary	91.00'	2.0" W x 2.0"	H Box Culvert	
					eadwall, Ke= 0.500
					1.00' S= 0.0000 '/' Cc= 0.900
			•	w Area= 0.03 sf	
#2	Primary	97.00'	20.0' long x !	5.0' breadth Broa	ad-Crested Rectangular Weir

Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00

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2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=4.20 cfs @ 13.10 hrs HW=97.20' (Free Discharge)

-1=Culvert (Barrel Controls 0.06 cfs @ 2.01 fps)

—2=Broad-Crested Rectangular Weir (Weir Controls 4.14 cfs @ 1.04 fps)

Summary for Pond 2P: POND 2

Inflow Area = 47.085 ac, 3.99% Impervious, Inflow Depth > 0.91" for 2-YEAR event

Inflow = 19.02 cfs @ 13.03 hrs, Volume= 3.572 af

Outflow = 14.15 cfs @ 13.51 hrs, Volume= 2.668 af, Atten= 26%, Lag= 28.8 min

Primary = 14.15 cfs @ 13.51 hrs, Volume= 2.668 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 95.14' @ 13.51 hrs Surf.Area= 20,417 sf Storage= 48,926 cf

Plug-Flow detention time= 102.4 min calculated for 2.659 af (74% of inflow)

Center-of-Mass det. time= 45.8 min (915.4 - 869.7)

38,597

98.00

Volume	Invert	Avail.S	torage	Storage	e Descri ption	
#1	87.60'	132,	284 cf	Custon	n Stage Data (Pri	smatic) Listed below (Recalc)
Elevation (feet)	Surf.	Area sq-ft)		:.Store c-feet)	Cum.Store (cubic-feet)	
87.60		64		0	0	
90.00	1	,200		1,517	1,517	
92.00	6	5,281		7,481	8,998	
94.00	13	3,968	2	20,249	29,247	
96.00	25	5,236	(39,204	68,451	

Device	Routing	Invert	Outlet Devices
#1	Primary	87.00'	3.0" W x 3.0" H Box Culvert
	•		L= 36.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 87.00' / 87.00' S= 0.0000 '/' Cc= 0.900
			n= 0.035, Flow Area= 0.06 sf
#2	Primary	94.50'	10.0' long x 5.0' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65
			2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

132,284

Primary OutFlow Max=14.13 cfs @ 13.51 hrs HW=95.14' (Free Discharge)

63,833

1=Culvert (Barrel Controls 0.19 cfs @ 3.08 fps)

-2=Broad-Crested Rectangular Weir (Weir Controls 13.94 cfs @ 2.16 fps)

Type III 24-hr 2-YEAR Rainfall=3.14"

Prepared by Hewlett-Packard Company

Printed 4/11/2017

HydroCAD® 10.00-16 s/n 01454 © 2015 HydroCAD Software Solutions LLC

Page 9

Summary for Link POA1: POA #1

Inflow Area = 2.271 ac, 2.38% Impervious, Inflow Depth > 1.01" for 2-YEAR event

Inflow = 1.82 cfs @ 12.33 hrs, Volume= 0.191 af

Primary = 1.82 cfs @ 12.33 hrs, Volume= 0.191 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Summary for Link POA2: POA #2

Inflow Area = 48.772 ac, 3.95% Impervious, Inflow Depth > 0.69" for 2-YEAR event

Inflow = 14.32 cfs @ 13.55 hrs, Volume= 2.799 af

Primary = 14.32 cfs @ 13.55 hrs, Volume= 2.799 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Solar Way Subdivision CUMBERLAND, MAINE

November 28, 2016 Revised February 6, 2017

INSPECTION AND MAINTENANCE OF STORMWATER MANAGEMENT FACILITIES

Cumberland Foreside Village Owner of Lot 9 will be responsible for the inspection and maintenance of all stormwater management facilities, the establishment of any contract services required to implement the program, and the keeping of records and maintenance log book as described herein. At a minimum, the appropriate and relevant activities for each of the stormwater management facilities should be performed on the prescribed schedule. Periodic inspection and maintenance of these site features and devices is necessary to prevent erosion and remove pollutants from stormwater runoff.

Inspections and Reports:

This project is located in the MS4 section of Town. The Homeowners are required to file an annual report noting the conditions of the site, including erosion issues and drainage related items.

SWALES, DITCHES, CURBS AND PAVED AREAS:

Swales, ditches, curbs and paved areas are easily inspected during a site walk or even a ride-by. Since visual inspection is easy, their condition should be assessed during and/or after significant rainfall events such as thunder showers and periods of heavy or extended rainfall and during periods of significant snowmelt. Any damage or unusual condition such as sedimentation of a ditch, erosion, damaged curb or dying vegetation should be recorded, dated and initialed by the inspector when observed. Even if there is no damage, the inspector should make record of these inspections at least twice annually.

Paved areas should be visually inspected monthly during the winter. The inspector should pay particular attention to the build up of sand around catch basin grates and remove accumulations that block the free flow of surface runoff to the catch basins. The date and initials of the inspector should be recorded on the forms provided as well as a notation of any cleanup effort that was made and the approximate volume of sand that was removed.

SOLAR WAY SUBDIVISION CUMBERLAND, MAINE

INSPECTION / MAINTENANCE LOG

SWALES, DITCHES, CURBS AND PAVED AREAS

I: INSPECTED - C: CLEANED - R: REPAIRED **DATE INITIALS ACTION COMMENT** 5/14/15 ABC I, C EXAMPLE: Remove sand deposits and debris as necessary.

STORMWATER MANAGEMENT SYSTEM MAINTENANCE PROGRAM SUMMARY CHECKLIST

			Freq	uency	
			Semi-		Long-
Item	Commentary	Month	Annual	Annual	Term
Open Swale, Ditches & Inlet Structures	Inspect for debris accumulation, erosion and excessive vegetation. Mow monthly, remove debris, repair and revegetate any area of erosion	X Mow		X	
Pavement	Review for damage and buildup of debris and sand.	X	X Sweep		

HOUSEKEEPNG REPORT

SOLAR WAY SUBDIVISION CUMBERLAND, MAINE

November 28, 2016

Housekeeping:

This project is located in a stream watershed. Protection of the groundwater quality is ensured by having good housekeeping practices and maintenance of the stormwater systems. Spill prevention must be incorporated into the plan. The project should follow the following steps:

 Building Owners should be made aware of possible groundwater and surface water contamination based on their actions. The spilling of products such as small engine fuel, cleaning products and paints need to be cleaned up. The use of fertilizers and pesticides should be done cautiously and in accordance with manufacturers recommendations.

The maintenance of the landscaping and parking lots should include the sweeping of the parking lots and removal of the materials that may cause dust.

During construction follow the erosion control measures outlined on the plans.

During construction, develop a waste handling program that identified potential contaminates that could be introduced to the aquifer. Follow hazardous waste rules if any items used are considered a hazardous waste. It is critical to the site that uncontrolled releases be prevented.

Oil absorbent pads should be used while refueling equipment.

This site may require dewatering of trenches. During construction, monitor stormwater runoff from the equipment and ground areas to minimize contamination of groundwater.

DEPARTMENT OF ENVIRONMENTAL PROTECTION

NRPA PERMIT BY RULE NOTIFICATION FORM (For use with DEP Regulation, Natural Resources Protection Act-Permit by Rule Standards, Chapter 305)

PLEASE TYPE OR PRIN	IT IN <i>BLACK INK ONL</i>	.Υ				
Name of Applicant: (owner)	Live Solar M	laine, LLC	Name of Age	nt:	Thomas S. (Greer, P.E.
Applicant Mailing Address:	1 Solar Way		Agent Phone area code):	# (include	207-781-524	12
Town/City:	Cumberland		PROJECT Inf Name of Tow		Cumberland	
State and Zip code:	ME 04021		Name of Wetl Waterbody:		Unnamed St	ream
Daytime Phone # (include area code):	207-749-811	17	Map #:	R04	Lot #:	24
Detailed Directions to	Site: I-295N, I	Exit 10, left onto	Bucknam Rd	, left on L	JS-1N, exit Tutt	le Rd, right on
to Tuttle Rd, 1st	right onto Middi	e Rd. 2nd left or	nto Greely rd,	right on	Hillside Ave2	4 miles on right.
			UTM Northing):	UTM Eas	7,473
Description of Project: Construction of two driveways over the stream with utilities.						
					3.	
Part of a larger proje (check one)→		After the Fact? ☐ Ye			ect 🛘 does (or) 🗎 do average low water).	pes not involve work
NRPA PERMIT BY RU				low water	average low water).	
I am filing notice of r						
Chapter 305. I and				of the stand	_	
Sec. (2) Act. Adj. to	Protected Natural Re	<u> </u>	ream Crossing ate Transportation	Engil [s/Permit Extension
Sec. (4) Replaceme			estoration of Natur		Sec. (18) Maintena Sec. (19) Activitie	
Sec. (5) REPEALED			&W Creation/Enhai		, ,	nal pool habitat
	of Rocks or Vegetation		nprovement	_	Sec. (20) Activitie	-
Sec. (7) Outfall Pipe	-	Sec. (14) RI	-			ate value inland water-
☐ Sec. (8) Shoreline stabilization ☐ Sec. (15) Public Boat Ramps fowl & wading bird habitat or shore-						
□ Sec. (9) Utility Crossing □ Sec. (16) Coastal Sand Dune Projects □ bird feeding & roosting areas						
NOTIFICATION FORMS CANNOT BE ACCEPTED WITHOUT THE NECESSARY ATTACHMENTS: Attach a check for the correct fee, payable to: "Treasurer, State of Maine". The current fee for NRPA PBR Notifications can be found at the Department's website: http://www.maine.gov/dep/feesched.pdf Attach a U.S.G.S. topo map or Maine Atlas & Gazetteer map with the project site clearly marked. Attach Proof of Legal Name if applicant is a corporation, LLC, or other legal entity. Provide a copy of Secretary of State's registration information (available at http://icrs.informe.org/nei-sos-icrs/ICRS?MainPage=x). Individuals and municipalities are not required to provide any proof of identity, Attach photos of the proposed site where activity will take place as required in PBR Sections checked above. Attach all other required submissions as outlined in the PBR Sections checked above.						
I authorize staff of th						
access the project site for the purpose of determining compliance with the rules. I also understand that this permit is not valid until approved by the Department or 14 days after receipt by the Department, whichever is less.						
By signing this Notification Form, I represent that the project meets all applicability requirements and standards in the rule and that the applicant has sufficient title, right, or interest in the property where the activity takes place.						
Signature of Agent or Applicant: Date: 3/27/17						
Keep a copy as a reco Environmental Protect of the DEP's receipt of years. Work carried AUGUSTA DEP 17 STATE HOUSE AUGUSTA, ME 0 (207)287-3901 OFFICE USE ONLY	ion at the appropriation at the appropriation. No furtional form of a FE STATION 34333-0017 FOR Ck.#	the regional office list ther authorization by D in standard is subject to the control of the co	ted below. The I EP will be issued ct to enforcement BANGOR 106 HOGA BANGOR, (207)941-4	DEP will sen after receip nt action. DEP N ROAD ME 04401	d a copy to the Tow t of notice. Permits PRESQUE ISL 1235 CENTRA PRESQUE ISL (207)764-0477 Staff	n Office as evidence are valid for two E DEP L DRIVE E, ME 04769
PBR# (03470	FP 75.00	O Date /	Acc. H	17/17	Def. Date	After Photos

SOLAR WAY SUBDIVISION

Hillside Ave, Cumberland, Maine

Owner/Applicant

LIVE LIFE MAINE, LLC 1 SOLAR WAY CUMBERLAND, ME 04021

Developer

LIVE LIFE MAINE, LLC 1 SOLAR WAY CUMBERLAND, ME 04021

Civil Engineer

PINKHAM & GREER CIVIL ENGINEERS
28 VANNAH AVENUE
PORTLAND, MAINE Ø41Ø3
207-781-5242

Surveyor

MAINE SURVEY CONSULTANTS, INC. P.O. BOX 485 HARRISON, ME 04040 207-583-6159

Wetlands Mapping & Soils Scientist

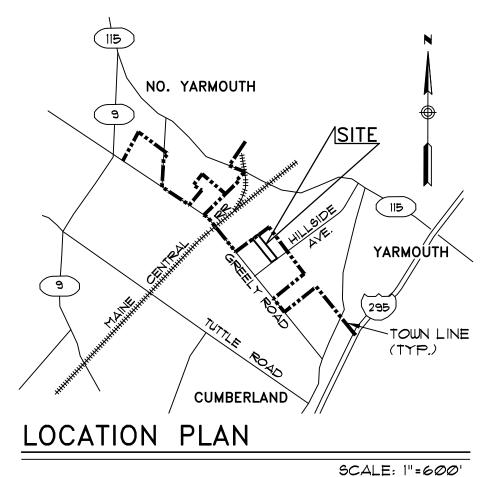
MARK HAMPTON \$ ASSOCIATES, INC. P.O. BOX 1931 PORTLAND, MAINE 04104-1931 201-713-8650

Hydrogeologist

DICK SWEET, SWEET ASSOCIATES 155 GRAY ROAD FALMOUTH, ME Ø41Ø5 2Ø7-797-211Ø

DRAWING LIST

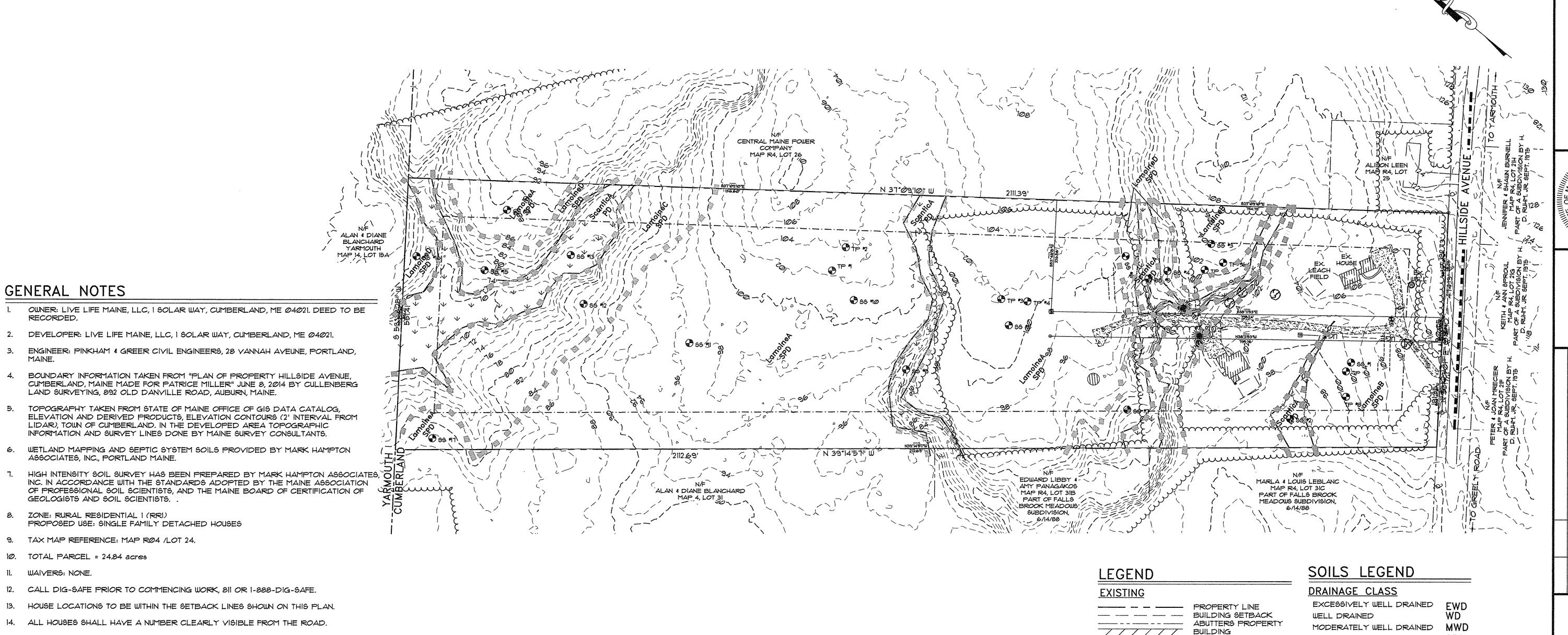
<u>SHEET</u>	TITLE
COVER	COVER
C1.Ø	EXISTING CONDITION PLAN
C1.1	SUBDIVISION PLAN
C1.2	SITE PLAN
Cl.3	NET RESIDENTIAL ACREAGE PLAN
C1.4	CONVENTIONAL LOT LAYOUT
C2.Ø	PLAN & PROFILE W/ EROSION CONTROL - SOLAR WAY
C3.Ø	EROSION CONTROL NOTES & DETAILS
C4.Ø	SITE DETAILS
D1.Ø	DRAINAGE ANALYSIS, EXISTING CONDITION
D2.Ø	DRAINAGE ANALYSIS, DEVELOPED CONDITION



5CALE: 1"=609

CURRENT SUBMITTAL: APRIL 11, 2017

SUBMITTED TO TOWN OF CUMBERLAND



14. ALL HOUSES SHALL HAVE A NUMBER CLEARLY VISIBLE FROM THE ROAD. 15. ALL NEW DWELLINGS IN THIS SUBDIVISION SHALL INCLUDE AN AUTOMATIC FIRE

LAND SURVEYING, 892 OLD DANVILLE ROAD, AUBURN, MAINE.

ASSOCIATES, INC., PORTLAND MAINE.

GEOLOGISTS AND SOIL SCIENTISTS.

ZONE: RURAL RESIDENTIAL I (RRI)

9. TAX MAP REFERENCE: MAP RØ4 /LOT 24.

10. TOTAL PARCEL = 24.84 acres

11. WAIVERS: NONE.

PROPOSED USE: SINGLE FAMILY DETACHED HOUSES

PROTECTION SPRINKLER SYSTEM CONFORMING TO APPLICABLE NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) STANDARDS AND APPROVED BY THE CUMBERLAND FIRE CHIEF.

16. COMMON OPEN SPACE FOR THE HOMEOWNERS IS INCLUDED IN THIS SUBDIVISION AND IS TO BE MAINTAINED BY THE HOMEOWNERS ASSOCIATION.

17. AN EASEMENT FOR PRIVATE PEDESTRIAN ACCESS TO THE COMMON OPEN SPACE IS TO BE MAINTAINED OVER LOT 5.

18. CONSTRUCTION WILL COMMENCE WITH FINAL APPROVAL AND AS LOTS ARE SOLD.

19. LOTS TO BE SERVICED BY INDIVIDUAL SEPTIC SYSTEMS AND WELLS. SEPTIC SYSTEMS MUST BE LOCATED AS SHOWN ON THIS PLAN OR AN ALTERNATE LOCATION APPROVED BY THE TOWN STAFF.

20. POWER, TELEPHONE AND CABLE SERVICES ARE TO BE UNDERGROUND FROM AN EXISTING POLE ON HILLSIDE AVENUE.

21. THE DEVELOPER WILL BE RESPONSIBLE FOR MAINTAINING THE ROAD, INCLUDING PLOWING, UNTIL SUCH TIME AS THE ROAD IS TAKEN OVER BY THE HOMEOWNERS ASSOCIATION.

22. ALL CONSTRUCTION AND SITE ALTERATIONS SHALL BE DONE IN ACCORDANCE WITH THE "MAINE EROSION AND SEDIMENT CONTROL BMPS" PUBLISHED BY THE BUREAU OF LAND AND WATER QUALITY, MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION, LATEST EDITION, MAY 2016.

23. IRON RODS TO BE SET AT ALL CORNERS OF LOTS BY THE DEVELOPER.

24. STREET NAME: SOLAR WAY

GENERAL NOTES

25. NO CONSTRUCTION OR FILLING OF WETLANDS OTHER THAN THAT SHOWN ON THE PLAN ARE ALLOWED. A NRPA PERMIT IS REQUIRED BY DEP.

26. ALL DRIVEWAY ACCESS WILL BE FROM WITHIN THE SUBDIVISION.

27. WELLS MUST BE LOCATED OUTSIDE OF THE "WELL EXCLUSION" ZONE SHOWN, WELL CASING MUST EXTEND A MINIMUM OF 15' BELOW THE BEDROCK SURFACE.

28. HIGH GROUNDWATER TABLES ARE A CONSIDERATION FOR MAINE SOILS. HOUSES WITH BASEMENTS ARE RECOMMENDED TO HAVE PROPER FOUNDATION DRAINAGE TO ACCOMMODATE THESE CONDITIONS.

///// BUILDING ---- WETLAND LIMIT V V WETLAND ZONING DISTRICT TOWN LINE BENCHMARK BOUND FOUND @IPF IRON PIPE FOUND **⊙**IPS IRON PIPE SET SEPTIC SYSTEM SOILS BOUNDARY

₽ тр ••

(3)

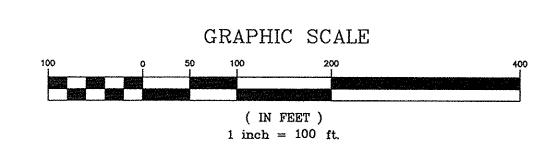
TEST PIT

TREE

SOMEWHAT POORLY DRAINED SPD PD POORLY DRAINED VPD VERY POORLY DRAINED

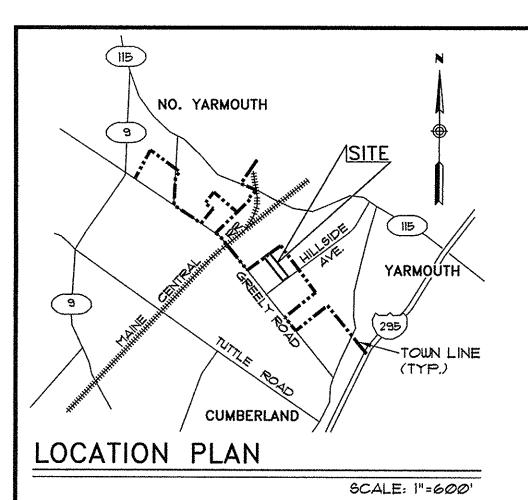
SLOPE DESIGNATION Ø-3% 3-8% 8-15% 15-25% 巴 25%

NOTE: HIGH INTENSITY SOIL SURVEY HAS BEEN PREPARED BY MARK HAMPTON ASSOCIATES, INC. IN ACCORDANCE WITH THE STANDARDS ADOPTED BY THE MAINE ASSOCIATION OF PROFESSIONAL SOIL SCIENTISTS, AND THE MAINE BOARD OF CERTIFICATION OF GEOLOGISTS AND SOIL SCIENTISTS.



LIVE LIFE 1 SOLAR \ CUMBERL SUBDIVISION
JE, CUMBERLAND, N

MAP/LOT R04 / 24

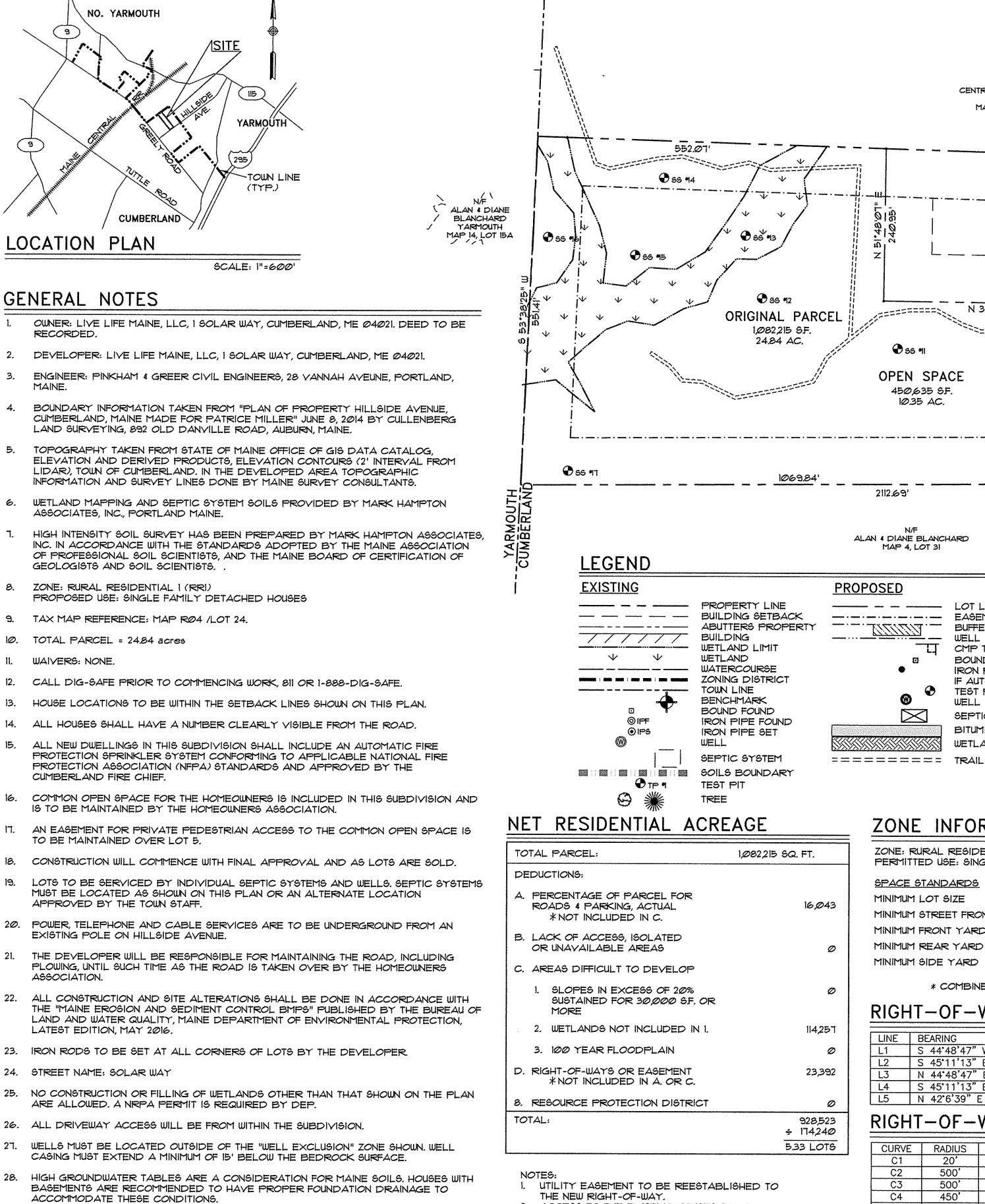


GENERAL NOTES

OWNER: LIVE LIFE MAINE, LLC, I SOLAR WAY, CUMBERLAND, ME 04021. DEED TO BE RECORDED.

ENGINEER: PINKHAM & GREER CIVIL ENGINEERS, 28 VANNAH AVEUNE, PORTLAND,

- 2. DEVELOPER: LIVE LIFE MAINE, LLC, I SOLAR WAY, CUMBERLAND, ME 04021.
- 4. BOUNDARY INFORMATION TAKEN FROM "PLAN OF PROPERTY HILLSIDE AVENUE, CUMBERLAND, MAINE MADE FOR PATRICE MILLER" JUNE 8, 2014 BY CULLENBERG LAND SURVEYING, 892 OLD DANVILLE ROAD, AUBURN, MAINE.
- TOPOGRAPHY TAKEN FROM STATE OF MAINE OFFICE OF GIS DATA CATALOG, ELEVATION AND DERIVED PRODUCTS, ELEVATION CONTOURS (2' INTERVAL FROM LIDAR), TOWN OF CUMBERLAND. IN THE DEVELOPED AREA TOPOGRAPHIC INFORMATION AND SURVEY LINES DONE BY MAINE SURVEY CONSULTANTS.
- 6. WETLAND MAPPING AND SEPTIC SYSTEM SOILS PROVIDED BY MARK HAMPTON ASSOCIATES, INC., PORTLAND MAINE.
- HIGH INTENSITY SOIL SURVEY HAS BEEN PREPARED BY MARK HAMPTON ASSOCIATES, INC. IN ACCORDANCE WITH THE STANDARDS ADOPTED BY THE MAINE ASSOCIATION OF PROFESSIONAL SOIL SCIENTISTS, AND THE MAINE BOARD OF CERTIFICATION OF GEOLOGISTS AND SOIL SCIENTISTS.
- 8. ZONE: RURAL RESIDENTIAL I (RRI)
- PROPOSED USE: SINGLE FAMILY DETACHED HOUSES 9. TAX MAP REFERENCE: MAP RØ4 /LOT 24.
- 10. TOTAL PARCEL = 24.84 acres
- II. WAIVERS: NONE.
- 12. CALL DIG-SAFE PRIOR TO COMMENCING WORK, 811 OR 1-888-DIG-SAFE.
- 13. HOUSE LOCATIONS TO BE WITHIN THE SETBACK LINES SHOWN ON THIS PLAN.
- 14. ALL HOUSES SHALL HAVE A NUMBER CLEARLY VISIBLE FROM THE ROAD.
- 15. ALL NEW DWELLINGS IN THIS SUBDIVISION SHALL INCLUDE AN AUTOMATIC FIRE PROTECTION SPRINKLER SYSTEM CONFORMING TO APPLICABLE NATIONAL FIRE PROTECTION ASSOCIATION (NFPA) STANDARDS AND APPROVED BY THE CUMBERLAND FIRE CHIEF.
- 16. COMMON OPEN SPACE FOR THE HOMEOWNERS IS INCLUDED IN THIS SUBDIVISION AND IS TO BE MAINTAINED BY THE HOMEOWNERS ASSOCIATION.
- 17. AN EASEMENT FOR PRIVATE PEDESTRIAN ACCESS TO THE COMMON OPEN SPACE IS TO BE MAINTAINED OVER LOT 5.
- 18. CONSTRUCTION WILL COMMENCE WITH FINAL APPROVAL AND AS LOTS ARE SOLD.
- MUST BE LOCATED AS SHOWN ON THIS PLAN OR AN ALTERNATE LOCATION APPROVED BY THE TOWN STAFF.
- 20. POWER, TELEPHONE AND CABLE SERVICES ARE TO BE UNDERGROUND FROM AN EXISTING POLE ON HILLSIDE AVENUE.
- 21. THE DEVELOPER WILL BE RESPONSIBLE FOR MAINTAINING THE ROAD, INCLUDING PLOWING, UNTIL SUCH TIME AS THE ROAD IS TAKEN OVER BY THE HOMEOWNERS ASSOCIATION.
- 22. ALL CONSTRUCTION AND SITE ALTERATIONS SHALL BE DONE IN ACCORDANCE WITH THE "MAINE EROSION AND SEDIMENT CONTROL BMPS" PUBLISHED BY THE BUREAU OF LAND AND WATER QUALITY, MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION. LATEST EDITION, MAY 2016,
- 23. IRON RODS TO BE SET AT ALL CORNERS OF LOTS BY THE DEVELOPER
- 24. STREET NAME: SOLAR WAY
- 25. NO CONSTRUCTION OR FILLING OF WETLANDS OTHER THAN THAT SHOWN ON THE PLAN ARE ALLOWED. A NRPA PERMIT IS REQUIRED BY DEP.
- 26. ALL DRIVEWAY ACCESS WILL BE FROM WITHIN THE SUBDIVISION.
- 27. WELLS MUST BE LOCATED OUTSIDE OF THE "WELL EXCLUSION" ZONE SHOWN, WELL CASING MUST EXTEND A MINIMUM OF 15' BELOW THE BEDROCK SURFACE.
- 28. HIGH GROUNDWATER TABLES ARE A CONSIDERATION FOR MAINE SOILS, HOUSES WITH BASEMENTS ARE RECOMMENDED TO HAVE PROPER FOUNDATION DRAINAGE TO ACCOMMODATE THESE CONDITIONS.
- 29. THE OWNER OF LOT 5 RESERVES THE RIGHT TO SUBDIVIDE LOT 5 TO CREATE TWO SINGLE FAMILY HOME LOTS IF THE ZONING CHANGES IN THE FUTURE. THE DIVISION WOULD HAVE TO MEET THE FUTURE ZONING ORDINANCE REQUIREMENTS, NO FURTHER SUBDIVISION OF LOTS I TO 4 IS ALLOWED.



2. ACCESS TO THE EXISTING HOUSES TO BE REESTABLISHED TO THE NEW RIGHT-OF-WAY.

ZONE INFORMATION

BUFFER

WELL EXCLUSION ZONE

BOUND TO BE SET

TEST PIT REQUIRED

BITUMINOUS PAVEMENT

SEPTIC FIELD

WETLAND IMPACT

CMP TRANSFORMER EASEMENT

IRON ROD W/CAP *IITT TO BE SET

IF AUTHORIZED BY THE DEVELOPER

ZONE: RURAL RESIDENTIAL I PERMITTED USE: SINGLE FAMILY DETACHED

CENTRAL MAINE POWER

COMPANY MAP R4, LOT 26

N 38°11'53" W

```<u>``</u>

PRIVATE 15' WIDE PEDESTRIAN

EASEMENT TO OPEN SPACE

FOR LOT OWNERS

N 39°14'57" W

SUBDIVISION PLAT

-========

OPEN SPACE

450,635 SF. 10.35 AC.

2112.69

MAP 4, LOT 31

SPACE STANDARDS CLUSTER RRI MINIMUM LOT SIZE 4 ACRES 60,000 SF. MINIMUM STREET FRONTAGE 200 FEET 100 FEET MINIMUM FRONT YARD 50 FEET 50 FEET MINIMUM REAR YARD 75 FEET 75 FEET

MINIMUM SIDE YARD 30 FEET \* 30 FEET \* \* COMBINED WIDTH AT LEAST 15 FEET

# RIGHT-OF-WAY LINE TABLE IMPERVIOUS INFORMATION

22,232 SF

±<u>35,938 SF</u>

13,706 SF

EXISTING IMPERVIOUS:

INCREASE OF:

PROPOSED IMPERVIOUS:

WETLAND IMPACTS 1007 Sq. Ft. B 203 Sq. Ft. C 1,419 Sq. Ft.

TOTAL: 2,629 Sq. Ft.

766 AC.

WETLAND IMPACT B

WETLAND IMPACT C-

EDWARD LIBBY

AMY PANAGAKOS

MAP R4, LOT 31B

PART OF FALLS

BROOK MEADOW

SUBDIVISION, 6/14/88

203 Sq. Ft.

1,419 Sq. Ft.

LINE	BEARING	DISTANCE
L1	S 44°48'47" W	30'
L2	S 45'11'13" E	50'
L3	N 44°48'47" E	30'
L4	S 45°11'13" E	30'
L5	N 42*6'39" E	50.03

RIGHT-OF-WAY CURVE TABLE

CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE
C1	20'	30.72'	27.79'	N 82'12'26" W	88*01'06"
C2	500'	65.45'	65.41'	N 34'26'52" W	7*30'02"
C3	500'	126.44'	126.11'	N 37'56'32" W	14'29'21"
C4	450'	113.8'	113.5'	N 37'56'32" W	14*29*21"
C5	550'	72'	71.95'	N 34'26'52" W	7*30'02"
C6	20'	32.11'	28.77'	N 07'47'34" E	91*58'54"

EXCEPTIONS: NO DETAILED SURVEY REPORT.

ITHE SEAL AND SIGNATURE OF DELMORE A. MAXFIELD, JR PLS ON THIS PLAN IS FOR THE DIMENSIONAL ACCURACY OF THE INTERIOR LOT LINES ONLY. CONFORMANCE OF THE ACTUAL STAKE OUT OF THE PROPOSED LOTS TO THIS PLAN WILL ONLY BE ENCOMPASSED BY SAID SEAL AND SIGNATURE IF PERFORMED UNDER THE DIRECTION OF SAID DELMORE A.

2)THIS SURVEY CONFORMS TO THE CURRENT STANDARDS ADOPTED BY THE MAINE BOARD OF LICENSURE FOR PROFESSIONAL LAND SURVEYORS, EXCEPTIONS AS NOTED ABOVE. 3NOT VALID OR TRUE COPY OF ORIGINAL UNLESS SIGNED AND EMBOSSED BY SURVEYOR PINKHAM & (

ALISON LEEN MAP R4, LOT

PARCEL 400 ACRES

174,288,70 SQ. F

38\*#53" E

STATE OF THE PARTY OF THE PARTY

TEMPORARY

DRIVEWAY

PLAN & PROFILE W/ EROSION CONTROL - SOLAR WAY

SCALE: 1"=200"

-WETLAND IMPACT /

65,468 SF

1,007 Sq. Ft.

MARLA & LOUIS LEBLANC

MAP R4, LOT 3IC

PART OF FALLS BROOK

MEADOWS SUBDIVISION,

GRAPHIC SCALE

( IN FEET )

1 inch = 100 ft

DRAWINGS INCLUDED IN THIS SUBMITTAL:

COVER

SITE PLAN

SITE DETAILS

EXISTING CONDITION PLAN

CONVENTIONAL LOT LAYOUT

NET RESIDENTIAL ACREAGE PLAN

EROSION CONTROL NOTES & DETAILS

DRAINAGE ANALYSIS, EXISTING CONDITION

DRAINAGE ANALYSIS, DEVELOPED CONDITION

SUBDIVISION PLAN, APPROVED BY THE

TOWN OF CUMBERLAND PLANNING BOARD

SUBDIVISION PLAN

1.7 AC.

9 37.0910" E

PARCEL B

20.84 ACRES

907,927.61 SQ. FT.

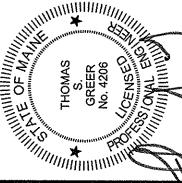
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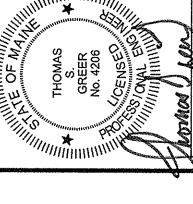
N 3914'57" W

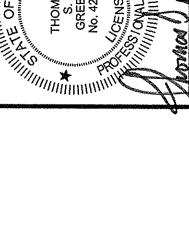
DATE:

EXISTING LOT LINES & EASEMENTS TO BE VACATED

131660







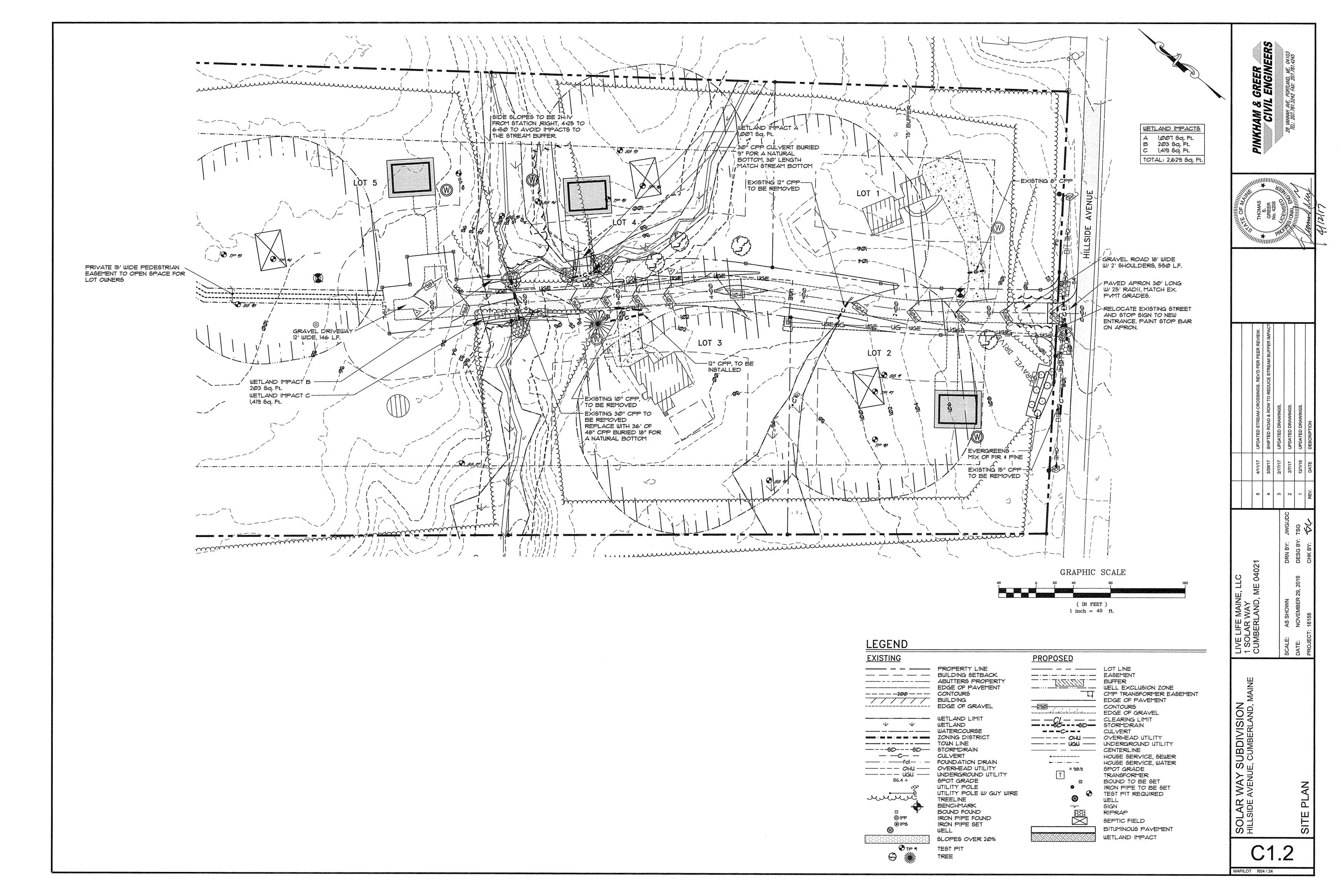
UPDATED STREAM CROSSINGS. REV'D PER PEER REVIEW.	SHIFTED ROAD & ROW TO REDUCE STREAM BUFFER IMPACT	ADDED GENERAL NOTE 29.	UPDATED DRAWINGS.	UPDATED DRAWINGS.	UPDATED DRAWINGS.	DESCRIPTION
4/11/17	3/28/17	212217	2/17/17	2/7/1/7	12/1/16	DATE

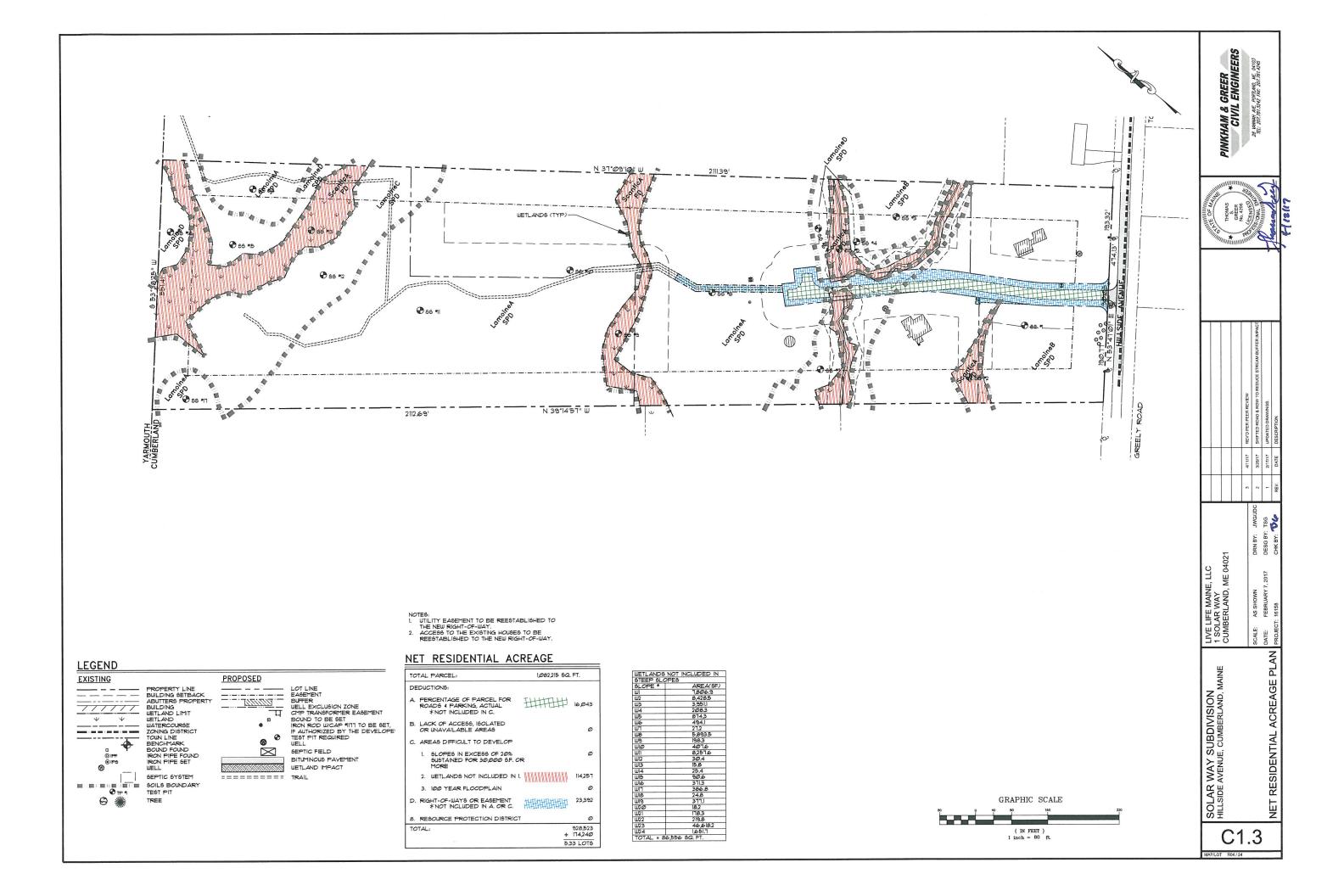
1 SOL,	1 SOLAR WAY		9	4/11/17	UPDATED
COMB	CUMBERLAND, ME 04021	21	9	3/28/17	SHIFTED
			4	212217	ADDED GE
			c	2112110	1 IBAATED
			3	71 // 17	טבואטייט
SCALE:	AS SHOWN	DRN BY: JWG/JDC	2	7 <i>NT\</i> 2	UPDATED
DATE:	NOVEMBER 29, 2016	DESG BY: TSG	<del>-</del>	12/1/16	UPDATED
PROJECT: 16158	. 16158	· · · · · · · · · · · · · · · · · · ·	ì	Li v C	10.000

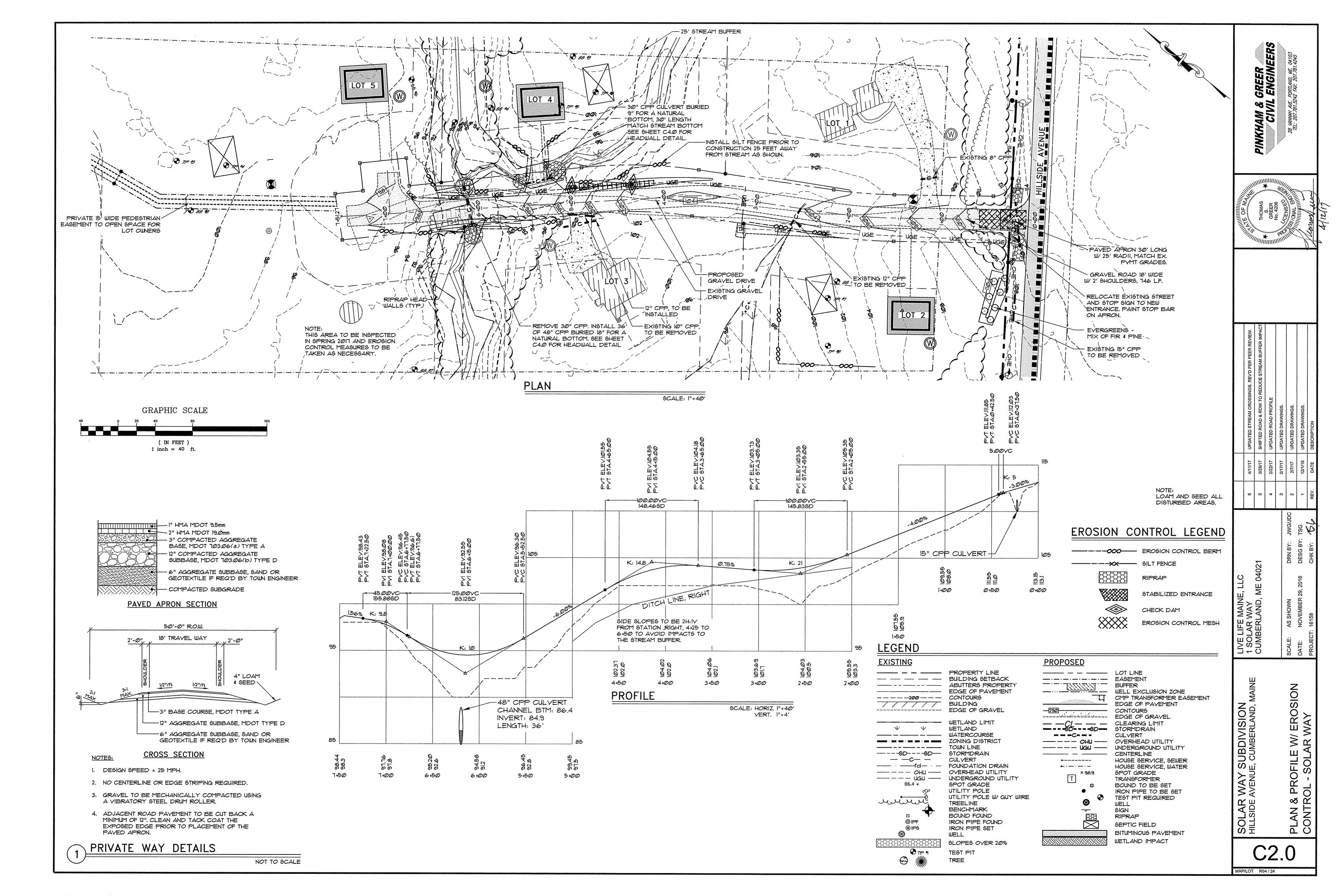
BDIVISION

MAP/LOT R04 / 24

DATE







# **EROSION CONTROL NOTES**

# **GENERAL:**

THE DRAWINGS DEPICT THE REQUIRED SOIL EROSION CONTROL MEASURES. THE CONTRACTOR IS RESPONSIBLE FOR MAINTAINING THE CONSTRUCTION SITE IN SUCH A MANNER THAT:

# I. SOIL EROSION IS KEPT TO A MINIMUM.

- 2. NO SEDIMENT LEAVES THE CONSTRUCTION SITE PROPER 3. ALL POSSIBLE MEASURES ARE EMPLOYED TO PREVENT SEDIMENT FROM ENTERING DRAINAGE COURSES AND WETLANDS EVEN BEYOND THE DETAILS SHOWN ON THIS PLAN IF NECESSARY.
- ALL EROSION CONTROL MEASURES SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH THE MAINE EROSION AND SEDIMENT 2. MATERIAL CONTROL BMPS PUBLISHED BY THE BUREAU OF LAND AND WATER QUALITY, MAINE DEPARTMENT OF ENVIRONMENTAL PROTECTION, MAY
- THE CONTRACTOR IS RESPONSIBLE FOR ALL FINES RESULTING FROM EROSION OR SEDIMENTATION FROM THE SITE TO SURROUNDING PROPERTIES, WATERBODIES, OR WETLAND AS A RESULT OF THIS PROJECT.
- LOAM AND SEED ALL DISTURBED AREAS AS SOON AS POSSIBLE AFTER DISTURBANCE, BUT NO LONGER THAN I DAYS. LOAM AND SEED ANY DISTURBED AREA WITHIN 15' OF WETLANDS OR WATERBODEIS WITHIN 48 HOURS OR PRIOR TO AND STORM EVENT. USE WINTER SEED RATES AND SPECIFICATIONS IF APPROPRIATE.
- INSPECT SOIL EROSION MEASURES WEEKLY AND AFTER SIGNIFICANT STORM EVENTS. MAKE ALL NECESSARY REPAIRS TO FACILITIES AS SOON AS POSSIBLE, BUT NO LONGER THAN 2 DAYS. CLEAN AND RESET SILT FENCES AND STONE CHECK DAMS WHICH ACCUMULATE SEDIMENT AND DEBRIS.
- PROTECT AND STABILIZE ALL AREAS NOT SCHEDULED FOR EROSION PREVENTION OR STABILIZATION BUT THAT SHOW SIGNS OF EROSION. NOTIFY OWNER OF ANY SIGNIFICANT EROSION PROBLEM.
- 6. APPLY MULCH TO BARE SOILS WITHIN I DAYS OF INITIAL DISTURBANCE OF SOILS, WITHIN 48 HOURS IF WITHIN 75' OF WETLAND OR WATERBODY, PRIOR TO ANY RAIN EVENT, OR PRIOR TO ANY WORK SHUTDOWN LASTING MORE THAN ONE DAY.
- TEMPORARILY SEED WITHIN I DAYS ANY AREA WHICH WILL BE LEFT DISTURBED AND UNWORKED FOR MORE THAN 14 DAYS WITH THE TEMPORARY SEED MIX LISTED BELOW. IF AREA IS WITHIN 75' OF A WETLAND OR WATERBODY, SEED WITHIN 48 HOURS. PERMANENTLY SEED ANY AREA WHICH CAN BE LOAMED AS SOON AS POSSIBLE WITH THE PERMANENT SEED MIX LISTED BELOW. DO NOT USE PERMANENT SEED MIX AFTER SEPTEMBER 15.
- 8. MULCH ALL AREAS SEEDED SO THAT SOIL IS NOT VISIBLE THROUGH THE MULCH REGARDLESS OF THE APPLICATION RATE. DURING THE GROWING SEASON (APRIL 15 - SEPT. 30) USE EROSION CONTROL MESH (OR MULCH AND NETTING) ON:
  - -THE BASE OF GRASSED WATERWAYS -SLOPES STEEPER THAN 15%
- -WITHIN 100 ft. OF STREAMS AND WETLANDS BETWEEN OCT. I AND APRIL 14 USE EROSION CONTROL MESH (OR MULCH AND NETTING) ON:
  - -SIDE SLOPES OF GRASSED WATERWAYS -SLOPES STEEPER THAN 8%
- 9. INSTALL EROSION CONTROL MESH IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. MESH TO BE EQUAL TO NORTH AMERICAN GREEN PRODUCT CI25BN,
- 10. FOLLOW SILT FENCE MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS FOR INSTALLATION OF SILT FENCE. SECURE ENTIRE BOTTOM OF FENCE EITHER BY BURYING BOTTOM OF FENCE IN A TRENCH OR BERMING WITH SOIL OR CHIPPED GRUBBINGS. REFER TO SILT FENCE DETAILS.
- PLACE AND GRADE LOAM IN A REASONABLY UNIFORM MANNER. WORK LIME AND FERTILIZER INTO THE SOIL TO A DEPTH OF 4 INCHES WITH A DISC, SPRING TOOTH HARROW OR OTHER SUITABLE EQUIPMENT. CONTINUE TILLAGE UNTIL A REASONABLY UNIFORM SEED BED IS PREPARED. REMOVE FROM SURFACE ALL STONES LARGER THAN 2" AND ALL OTHER UNSUITABLE MATERIAL. LIME AND FERTILIZER SHOULD BE MIXED INTO SOIL PRIOR TO ROLLING EXCEPT IF INCLUDED IN HYDROSEED MIXTURE, PERMANENT STABLILIZATION OF REVEGETATED AREAS IS CONSIDERED AS 90% CATCH.
- 12. ALL CULVERT OR PIPE OUTFALL PROTECTION MUST BE INSTALLED WITHIN 48 HOURS OF INSTALLING NEW PIPE OR CULVERT.
- 13. DITCHES AND CHANNELS DESIGNATED TO BE LINED WITH RIPRAP AND/OR EROSION CONTROL MESH MUST BE INSTALLED WITHIN 48 HOURS OF COMPLETING THE GRADING OF THAT SECTION OF DITCH OR CHANNEL
- 14. ALL CATCH BASINS, NEW OR EXISTING, THAT MAY RECEIVE RUNOFF FROM DISTURBED AREAS MUST BE PROTECTED BY INSTALLING AND MAINTAINING SILT SACKS DURING CONSTRUCTION.
- 15. WATER FROM CONSTRUCTION TRENCH DEWATERING OR TEMPORARY STREAM DIVERSION WILL PASS FIRST THROUGH A FILTER BAG OR SECONDARY CONTAINMENT STRUCTURE (E.G. HAY BALE OR EROSION CONTROL MIX LINED POOL) PRIOR TO DISCHARGE. THE DISCHARGE SITE SHALL BE SELECTED TO AVOID FLOODING, ICING, AND SEDIMENT DISCHARGES TO A PROTECTED RESOURCE. IN NO CASE SHALL THE FILTER BAG OR CONTAINMENT STRUCTURE BE LOCATED WITHIN 100 FEET OF A PROTECTED NATURAL RESOURCE.

# **TOPSOIL**

SUITABLE TOPSOIL SALVAGED FROM SITE OR SCREENED, LOOSE AND FRIABLE SANDY LOAM OR LOAM AS DEFINED BY THE USDA SOIL CONSERVATION SERVICE CLASSIFICATION SYSTEM, FREE FROM ADMIXTURE OF SUBSOIL, REFUSE, LARGE STONES, CLODS, ROOTS, WEEDS RHIZOMES OR OTHER UNDESIREABLE FOREIGN MATTER AS DETERMINED BY THE INSPECTING AUTHORITY. CONTRACTOR SHALL SUBMIT REPORTS OF LOAM TEST RESULTS PERFORMED BY AN INDEPENDENT TESTING LABORATORY FOR TOPSOIL FROM DIFFERENT SOURCES PRIOR TO PLACING. THE COST OF TESTING SHALL BE INCIDENTAL TO THE COST OF TOPSOIL TOPSOIL SHALL MEET THE FOLLOWING SPECIFICATIONS:

SAND - 0.08 IN. TO 0.002 IN. DIAMETER (% BY VOLUME)	45	- 7
SILT - 0.002 IN. TO 0.00008 IN. DIAMETER (% BY VOLUME)	20	- 4
CLAY - LESS THAN 0.00008 IN. DIAMETER (% BY VOLUME)	5	- 1

ORGANICS (SHALL MEET THE REQUIREMENTS OF MOOT STANDARD SPECIFICATION 117.09 PEAT HUMUS) (% BY VOLUME). 10 - 20

CALCIUM (CA) (% SATURATION)	60 - 80
MAGNESIUM (MG) (% SATURATION)	10 - 25
POTASSIUM (K) (% SATURATION)	. 2.1 - 3.0
PHOSPHORUS (P) (POUNDS/ACRE)	10 - 40
PH	6.0 - 6.5

# MAXIMUM STONE SIZE (INCHES)..........3/4

PERMEABILITY (INCHES PER HOUR)....3 - 10

USE PERMANENT SEED MIXES AND RATES BETWEEN 5/15 AND 9/30. USE TEMPORARY SEED MIXES FOR PERIODS LESS THAN 12 MONTHS. IF USING TEMPORARY SEED MIXES AND RATES BETWEEN 10/1 AND 5/14, RE-SEED WITH PERMANENT SEED MIX AFTER 5/15.

# PERMANENT SEED:

MDOT 117,03(a) METHOD NUMBER 3

# TEMPORARY SEED:

OATS	80.00 LBS/ACRE	4/01 - 5/14
ANNUAL RYEGRASS	40.00 LBS/ACRE	
SUDANGRASS	40.00 LBS/ACRE	5/15 - 8/14
ANNUAL RYEGRASS	80.00 LBS/ACRE	5/15 - 9/14
WINTER RYE	112.00 LBS/ACRE	9/15 - 9/30
WINTER RYE (W/ MULCH COVER)	. 112.00 LBS/ACRE	10/01 - 3/31

# LIME AND FERTILIZER:

APPLY GROUND LIMESTONE (EQUIVALENT TO 50% CALCIUM PLUS MAGNESIUM OXIDE) AT A RATE OF 3 TONS PER ACRE (138 POUNDS PER 1000 SQUARE FEET). APPLY FERTILIZER (10-20-20) AT A RATE OF 800 POUNDS PER ACRE (18.4 POUNDS PER 1000 SQUARE FEET).

MULCH:		
STRAW OR HAY (ANCHORED)	. 185 - 275 LBS	PROTECTED AREA WINDY AREAS
SHREDDED OR CHOPPED		MODERATE TO HIG
		VELOCITY AREAS
EXCELSIOR MAT	AS REQUIRED	STEEP SLOPES
MULCH ANCHORING		
PEG AND TWINE		TIRES
ASPHALT EMULSION		FIDER

# **HOUSEKEEPING**

LAY CULVERT

MATCH DITCHLINE

INVERTS TO

THIS PROJECT IS LOCATED IN A STREAM WATERSHED. PROTECTION OF THE GROUNDWATER QUALITY IS ENSURED BY HAVING GOOD HOUSEKEEPING PRACTICES AND MAINTENANCE OF THE STORMWATER SYSTEMS, SPILL PREVENTION MUST BE INCORPORATED INTO THE PLAN. THE PROJECT SHOULD FOLLOW THE FOLLOWING STEPS:

HOMEOWNERS SHOULD BE MADE AWARE OF POSSIBLE GROUNDWATER AND SURFACE WATER CONTAMINATION BASED ON THEIR ACTIONS. THE SPILLING OF PRODUCTS SUCH AS SMALL ENGINE FUEL, CLEANING PRODUCTS AND PAINTS NEED TO BE CLEANED UP. THE USE OF FERTILIZERS AND PESTICIDES SHOULD BE DONE CAUTIOUSLY AND IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

THE MAINTENANCE OF THE LANDSCAPING AND PARKING AREAS SHOULD INCLUDE THE SWEEPING OF THE PARKING AREAS AND REMOVAL OF THE MATERIALS THAT MAY CAUSE DUST.

DURING CONSTRUCTION FOLLOW THE EROSION CONTROL MEASURES OUTLINED IN THE DESIGN DRAWINGS.

DURING CONSTRUCTION, DEVELOP A WASTE HANDLING PROGRAM THAT IDENTIFIES POTENTIAL CONTAMINATES THAT COULD BE INTRODUCED TO THE AQUIFER. FOLLOW HAZARDOUS WASTE RULES IF ANY ITEMS USED ARE CONSIDERED A HAZARDOUS WASTE. IT IS CRITICAL TO THE SITE THAT UNCONTROLLED RELEASES BE PREVENTED.

OIL ABSORBENT PADS SHOULD BE USED WHILE REFUELING EQUIPMENT.

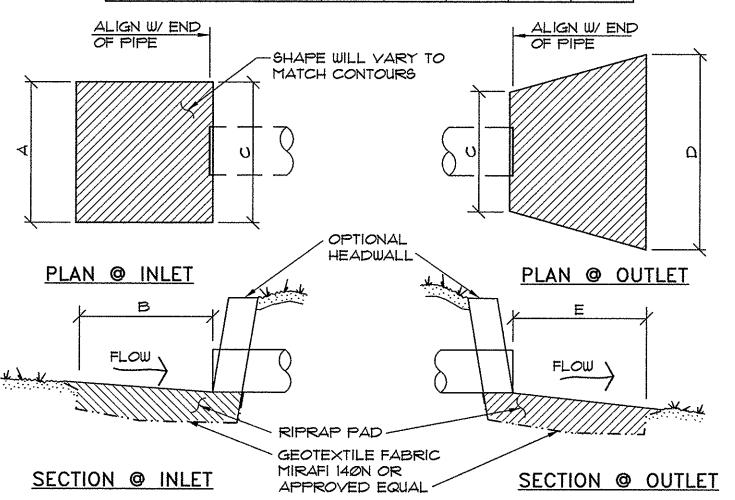
THIS SITE MAY REQUIRE DEWATERING OF TRENCHES. DURING CONSTRUCTION, MONITOR STORMWATER RUNOFF FROM THE EQUIPMENT AND GROUND AREAS TO MINIMIZE CONTAMINATION OF GROUNDWATER.

# STABILIZING SITE FOR THE WINTER:

- 1. STANDARD CONDITIONS REQUIRING THE TIMELY STABILIZATION OF DITCHES AND CHANNELS THE CONTRACTOR WILL CONSTRUCT AND STABILIZE ALL STONE-LINED DITCHES AND CHANNELS ON THE SITE BY NOVEMBER 1. THE CONTRACTOR WILL CONSTRUCT AND STABILIZE ALL GRASS-LINED DITCHES AND CHANNELS ON THE SITE BY SEPTEMBER 15. IF THE CONTRACTOR FAILS TO STABILIZE A DITCH OR CHANNEL TO BE GRASS-LINED BY SEPTEMBER 15, THEN THE CONTRACTOR WILL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE DITCH FOR LATE FALL AND WINTER:
- a. INSTALL A SOD LINING IN THE DITCH THE CONTRACTOR WILL LINE THE DITCH WITH PROPERLY INSTALLED SOD BY OCTOBER I. PROPER INSTALLATION INCLUDES THE CONTRACTOR PINNING THE SOD ONTO THE SOIL WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, WATERING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL, AND ANCHORING THE SOD WITH JUTE OR PLASTIC MESH TO PREVENT THE SOD STRIPS FROM SLOUGHING DURING FLOW CONDITIONS.
- D. INSTALL A STONE LINING IN THE DITCH THE CONTRACTOR WILL LINE THE DITCH WITH STONE RIPRAP BY NOVEMBER 1. THE CONTRACTOR WILL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE AND LINING THICKNESS NEEDED TO WITHSTAND THE ANTICIPATED FLOW VELOCITIES AND FLOW DEPTHS WITHIN THE DITCH. IF NECESSARY, THE CONTRACTOR WILL REGRADE THE DITCH PRIOR TO PLACING THE STONE LINING SO TO PREVENT THE STONE LINING FROM REDUCING THE DITCH'S CROSS-SECTIONAL AREA.
- 2. STANDARD CONDITIONS REQUIRING THE TIMELY STABILIZATION OF DISTURBED SLOPES THE CONTRACTOR WILL CONSTRUCT AND STABILIZE STONE-COVERED SLOPES BY NOVEMBER 1. THE CONTRACTOR WILL SEED AND MULCH ALL SLOPES TO BE VEGETATED BY SEPTEMBER 15. ANY AREA HAVING A GRADE GREATER THAN 15% (10H:1V) IS A SLOPE. IF THE CONTRACTOR FAILS TO STABILIZE ANY SLOPE TO BE VEGETATED BY SEPTEMBER IS, THEN THE CONTRACTOR WILL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE SLOPE FOR LATE FALL AND WINTER:
- a. STABILIZE THE SOIL WITH TEMPORARY VEGETATION AND EROSION CONTROL MATS BY OCTOBER THE CONTRACTOR WILL SEED THE DISTURBED SLOPE WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1,000 SQUARE FEET AND APPLY EROSION CONTROL MATS OVER THE MULCHED SLOPE. THE CONTRACTOR WILL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS. IF THE RYE FAILS TO GROW AT LEAST THREE INCHES OR COVER AT LEAST 15% OF THE DISTURBED SLOPE BEFORE NOVEMBER 1, THEN THE CONTRACTOR WILL COVER THE SLOPE WITH A LAYER OF WOODWASTE COMPOST AS DESCRIBED IN ITEM C OF THIS CONDITION OR WITH STONE RIPRAP AS DESCRIBED IN ITEM D OF THIS CONDITION.
- b. STABILIZE THE SLOPE WITH SOD THE CONTRACTOR WILL STABILIZE THE DISTURBED SLOPE WITH PROPERLY INSTALLED SOD BY OCTOBER I. PROPER INSTALLATION INCLUDES THE CONTRACTOR PINNING THE SOD ONTO THE SLOPE WITH WIRE PINS, ROLLING THE SOD TO GUARANTEE CONTACT BETWEEN THE SOD AND UNDERLYING SOIL, AND WATERING TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL. THE CONTRACTOR WILL NOT USE LATE-SEASON SOD INSTALLATION TO STABILIZE SLOPES HAVING A GRADE GREATER THAN 33% (3H:IV).
- C. STABILIZE THE SLOPE WITH WOODWASTE COMPOST THE CONTRACTOR WILL PLACE A SIX-INCH LAYER OF WOODWASTE COMPOST ON THE SLOPE BY NOVEMBER I. PRIOR TO PLACING THE WOODWASTE COMPOST, THE CONTRACTOR WILL REMOVE ANY SNOW ACCUMULATION ON THE DISTURBED SLOPE. THE CONTRACTOR WILL NOT USE WOODWASTE COMPOST TO STABILIZE SLOPES HAVING GRADES GREATER THAN 50% (2H:IV) OR HAVING GROUNDWATER SEEPS ON THE SLOPE FACE.
- d. STABILIZE THE SLOPE WITH STONE RIPRAP THE CONTRACTOR WILL PLACE A LAYER OF STONE RIPRAP ON THE SLOPE BY NOVEMBER I. THE CONTRACTOR WILL HIRE A REGISTERED PROFESSIONAL ENGINEER TO DETERMINE THE STONE SIZE NEEDED FOR DRAINAGE AND SOIL SEPARATION.
- 3. STANDARD CONDITIONS REQUIRING THE TIMELY STABILIZATION OF DISTURBED SOILS BY SEPTEMBER 15 THE CONTRACTOR WILL SEED AND MULCH ALL DISTURBED SOILS ON AREAS HAVING A SLOPE LESS THAN 15%. IF THE CONTRACTOR FAILS TO STABILIZE THESE SOILS BY THIS DATE, THEN THE CONTRACTOR WILL TAKE ONE OF THE FOLLOWING ACTIONS TO STABILIZE THE SOIL FOR LATE FALL AND WINTER:
- a. STABILIZING THE SOIL WITH TEMPORARY VEGETATION BY OCTOBER I THE CONTRACTOR WILL SEED THE DISTURBED SOIL WITH WINTER RYE AT A SEEDING RATE OF 3 POUNDS PER 1,000 SQUARE FEET, AND ANCHOR THE MULCH WITH PLASTIC NETTING. THE CONTRACTOR WILL MONITOR GROWTH OF THE RYE OVER THE NEXT 30 DAYS, IF THE RYE FAILS TO GROW AT LEAST THREE INCHES OR COVER AT LEAST 15% OF THE DISTURBED SOIL BEFORE NOVEMBER I, THEN THE CONTRACTOR WILL MULCH THE AREA FOR OVER-WINTER PROTECTION AS DESCRIBED IN ITEM C OF THIS STANDARD CONDITION.
- b. STABILIZE THE SOIL WITH SOD THE CONTRACTOR WILL STABILIZE THE DISTURBED SOIL WITH PROPERLY INSTALLED SOD BY OCTOBER I. PROPER INSTALLATION INCLUDES THE CONTRACTOR PINNING THE SOD ONTO THE SOIL WITH WIRE PINS, ROLLING THE SOD TO PROMOTE ROOT GROWTH INTO THE DISTURBED SOIL.
- C. STABILIZE THE SOIL WITH MULCH BY NOVEMBER I THE CONTRACTOR WILL MULCH THE DISTURBED SOIL BY SPREADING HAY OR STRAW AT A RATE OF AT LEAST 150 POUNDS PER 1,000 SQUARE FEET ON THE AREA SO THAT NO SOIL IS VISIBLE THROUGH THE MULCH. PRIOR TO APPLYING THE MULCH, THE CONTRACTOR WILL REMOVE ANY SNOW ACCUMULATION ON THE DISTURBED AREA. IMMEDIATELY AFTER APPLYING THE MULCH, THE CONTRACTOR WILL ANCHOR THE MULCH WITH PLASTIC NETTING TO PREVENT WIND FROM MOVING THE MULCH OFF THE DISTURBED SOIL.

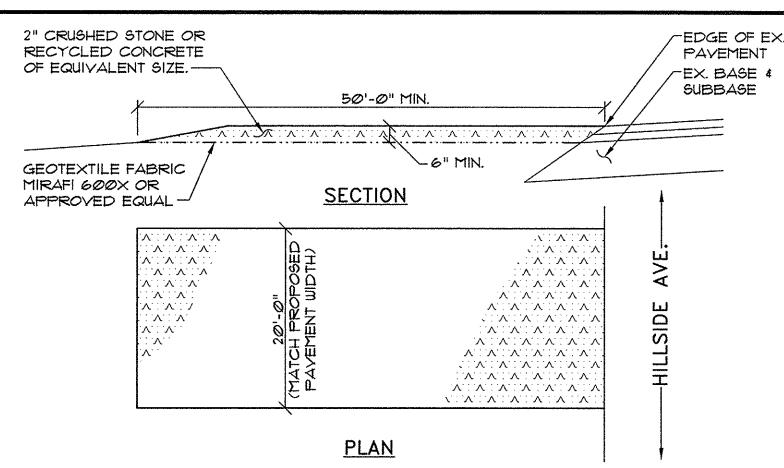
# RIPRAP PAD MUST BE INSTALLED WITHIN 48 HOURS OF INSTALLING NEW PIPE OR CULVERT.

PIPE	A	В	С	D	E	PAD DEPTH	D50
UDSF OUTLETS			3 FT.	3 FT.	3 FT.	10"	4"
12"¢	3 FT.	2 FT.	3 FT.	9 FT.	8 FT.	15"	6"
15"¢	3.75 FT.	2.5 FT.	3.75 FT.	11.5 FT.	10 FT.	15"	6"
24"¢	4 FT.	3 FT.	4 FT.	14.5 FT.	12 FT.	24"	8"



RIPRAP PADS & HEADWALLS DETAILS

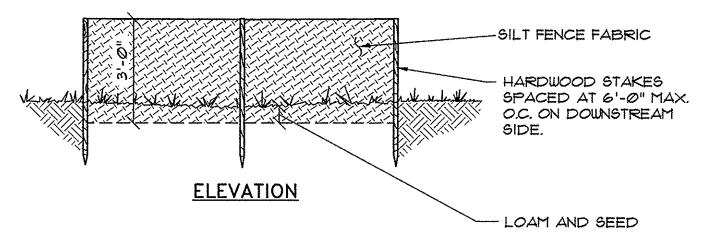
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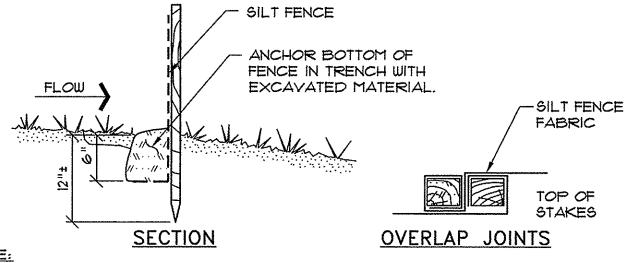


- I. MAINTAIN ENTRANCE IN A CONDITION THAT WILL PREVENT TRACKING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAY. IF WASHING IS REQUIRED PREVENT SEDIMENT FROM ENTERING WATERWAYS, DITCHES OR STORM DRAINS.
- 2. REMOVE STABILIZED CONSTRUCTION ENTRANCE TO FINISH ROAD CONSTRUCTION 4 PAVEMENT.

# STABILIZED CONSTRUCTION ENTRANCE DETAIL

NOT TO SCALE



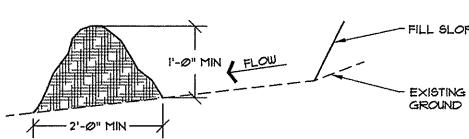


CONTRACTORS OPTION TO USE SEDIMENT BARRIER OR SILT FENCE FOR SLOPE PROTECTION.

NOTES:

F. PH: 50 - 80

SILT FENCE



1. EROSION CONTROL MIX CAN BE MANUFACTURED ON OR OFF THE SITE. IT MUST CONSIST PRIMARILY OF ORGANIC MATERIAL SEPARATED AT THE POINT OF GENERATION, AND MAY INCLUDE: SHREDDED BARK, STUMP GRINDINGS, COMPOSTED BARK, OR FLUME GRIT AND FRAGMENTED WOOD GENERATED FROM WATER-FLUME LOG HANDLING

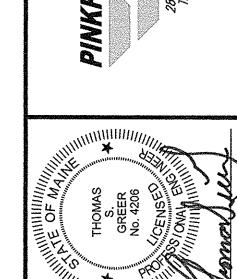
WOOD CHIPS. GROUND CONSTRUCTION DEBRIS, REPROCESSED WOOD PRODUCTS OR BARK CHIPS WILL NOT BE ACCEPTABLE AS THE ORGANIC COMPONENT OF THE MIX. EROSION CONTROL MIX SHALL CONTAIN A WELL-GRADED MIXTURE OF PARTICLE SIZES AND MAY CONTAIN ROCKS LESS THAN 4" IN DIAMETER EROSION CONTROL MIX MUST BE FREE OF REFUSE, PHYSICAL CONTAMINANTS, AND MATERIAL TOXIC TO PLANT

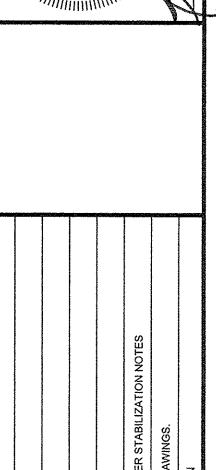
THE MIX COMPOSITION SHALL MEET THE FOLLOWING STANDARDS: A. ORGANIC MATERIAL: BETWEEN 20% - 100% (DRY WEIGHT BASIS) B. PARTICLE SIZE: BY WEIGHT, 100% PASSING 6" SCREEN, 70-85% PASSING 0.75" SCREEN C. THE ORGANIC PORTION NEEDS TO BE FIBROUS AND ELONGATED. D. LARGE PORTIONS OF SILTS, CLAYS OR FINE SANDS ARE NOT ACCEPTABLE IN THE MIX.

E. SOLUBLE SALTS CONTENT SHALL BE LESS THAN 40 MMHOS/CM.

- 2. ON SLOPES LESS THAN 5% OR AT THE BOTTOM OF SLOPES 2:1 OR LESS UP TO 20 FEET LONG, THE BARRIER MUST CONFORM TO THE ABOVE DIMENSIONS. ON THE LONGER OR STEEPER SLOPES, THE BARRIER SHOULD BE WIDER TO ACCOMMODATE THE ADDITIONAL FLOW.
- 3. THE BARRIER MUST BE PLACED ALONG A RELATIVELY LEVEL ELEVATION. IT MAY BE NECESSARY TO CUT TALL GRASSES OR WOODY VEGETATION TO AVOID CREATING VOIDS AND BRIDGES THAT WOULD ENABLE FINES TO WASH UNDER THE BARRIER THROUGH THE GRASS BLADES OR PLANT STEMS.
- 4. LOCATIONS WHERE OTHER BMP'S SHOULD BE USED: A. AT LOW POINTS OF CONCENTRATED FLOW
  - B. BELOW CULVERT OUTLET APRONS C. WHERE A PREVIOUS STAND-ALONE EROSION CONTROL MIX APPLICATION HAS FAILED D. AT THE BOTTOM OF STEEP PERIMETER SLOPES THAT ARE MORE THAN 50 FEET FROM TOP TO BOTTOM (LARGE UPGRADIENT WATERSHED) E. AROUND CATCH BASING AND CLOSED STORM DRAIN SYSTEMS.
- 5. THE EROSION CONTROL MIX BARRIERS SHOULD BE INSPECTED REGULARLY AND AFTER EACH LARGE RAINFALL. REPAIR ALL DAMAGED SECTIONS OF BERM IMMEDIATELY BY REPLACING OR ADDING ADDITIONAL MATERIAL PLACED ON THE BERM TO THE DESIRED HEIGHT AND WIDTH.
- 6. IT MAY BE NECESSARY TO REINFORCE THE BARRIER WITH SILT FENCE OR STONE CHECK DAMS IF THERE ARE SIGNS OF UNDERCUTTING OR THE IMPOUNDMENT OF LARGE VOLUMES OF WATER
- 7. SEDIMENT DEPOSITS SHOULD BE REMOVED WHEN THEY REACH APPROXIMATELY ONE-HALF THE HEIGHT OF THE
- 8. REPLACE SECTIONS OF BERM THAT DECOMPOSE, BECOME CLOGGED WITH SEDIMENT OR OTHERWISE BECOME INEFFECTIVE. THE BARRIER SHOULD BE RESHAPED AS NEEDED. 9. EROSION CONTROL MIX BARRIERS CAN BE LEFT IN PLACE AFTER CONSTRUCTION. ANY SEDIMENT DEPOSITS
- REMAINING IN PLACE AFTER BARRIER IS NO LONGER REQUIRED SHOULD BE SPREAD TO CONFORM TO THE EXISTING GRADE AND BE SEEDED AND MULCHED. WOODY VEGETATION CAN BE PLANTED INTO THE BARRIERS, OR THEY CAN BE OVER-SEEDED WITH LEGUMES. IF THE BARRIER NEEDS TO BE REMOVED, IT CAN BE SPREAD OUT

EROSION CONTROL MIX SEDIMENT BARRIER SURFACE DRAINAGE SEDIMENT CONTROL EER





SUBDIVISION, CUMBERLAND, N

I CONTROI DETAILS EROSION NOTES & I  $O \exists$ 

TYPICAL DRIVEWAY CULVERT DETAIL

DRIVEWAY CULVERT

NOT TO SCALE

—2' MIN. COVER (TYP)

NOT TO SCALE

MAP/LOT R04 / 24

# TRAIL DESIGN NOTES

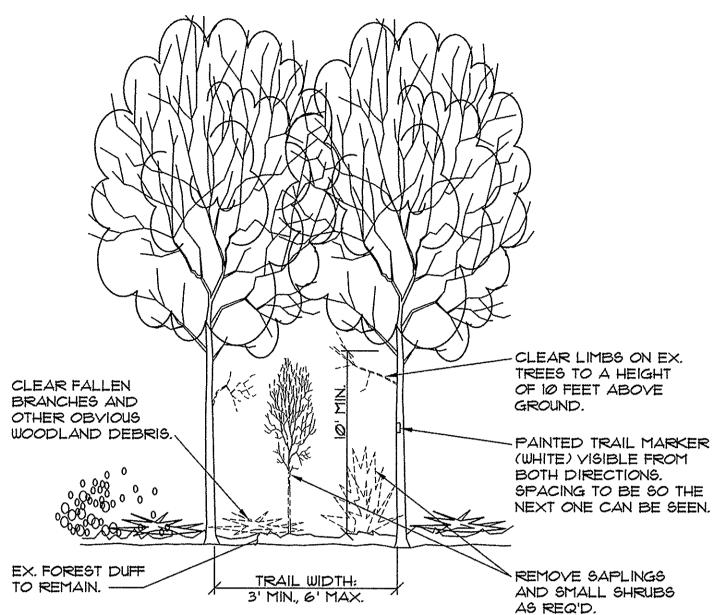
- PROPOSED TRAILS SHALL CONFORM TO THE FOLLOWING STANDARDS:

  A. TRAIL IMPROVEMENTS SHALL DEMONSTRATE ADHERENCE TO PRINCIPLES OF
- QUALITY TRAIL DESIGN.

  B. TRAILS SHALL HAVE A VERTICAL CLEARANCE OF NOT LESS THAN TEN (10) FEET.

  C. THE WIDTH OF THE TRAIL SURFACE MAY VARY DEPENDING UPON TYPE OF USE TO BE ACCOMMODATED, BUT IN NO CASE SHALL IT BE LESS THAN THREE (3) FEET
- OR GREATER THAN SIX (6) FEET.

  D. NO TRAIL SHALL BE DESIGNED WITH THE INTENT TO ACCOMMODATE MOTORIZED VEHICLES.

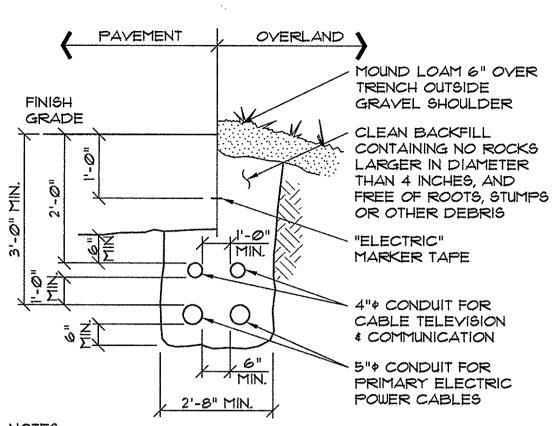


TES: THE TRAIL IS PRIVATE FOR THE

1. THE TRAIL IS PRIVATE FOR THE 5 HOME OWNERS AND THEIR GUESTS.
2. MINOR GRADING AND SHAPING OF THE TRAIL IS REQUIRED TO REMOVE RUTS.
3. ALL AREAS DISTURBED NEED TO BE LOAMED AND SEEDED.

WOODS TRAIL SECTION

NOT TO SCALE



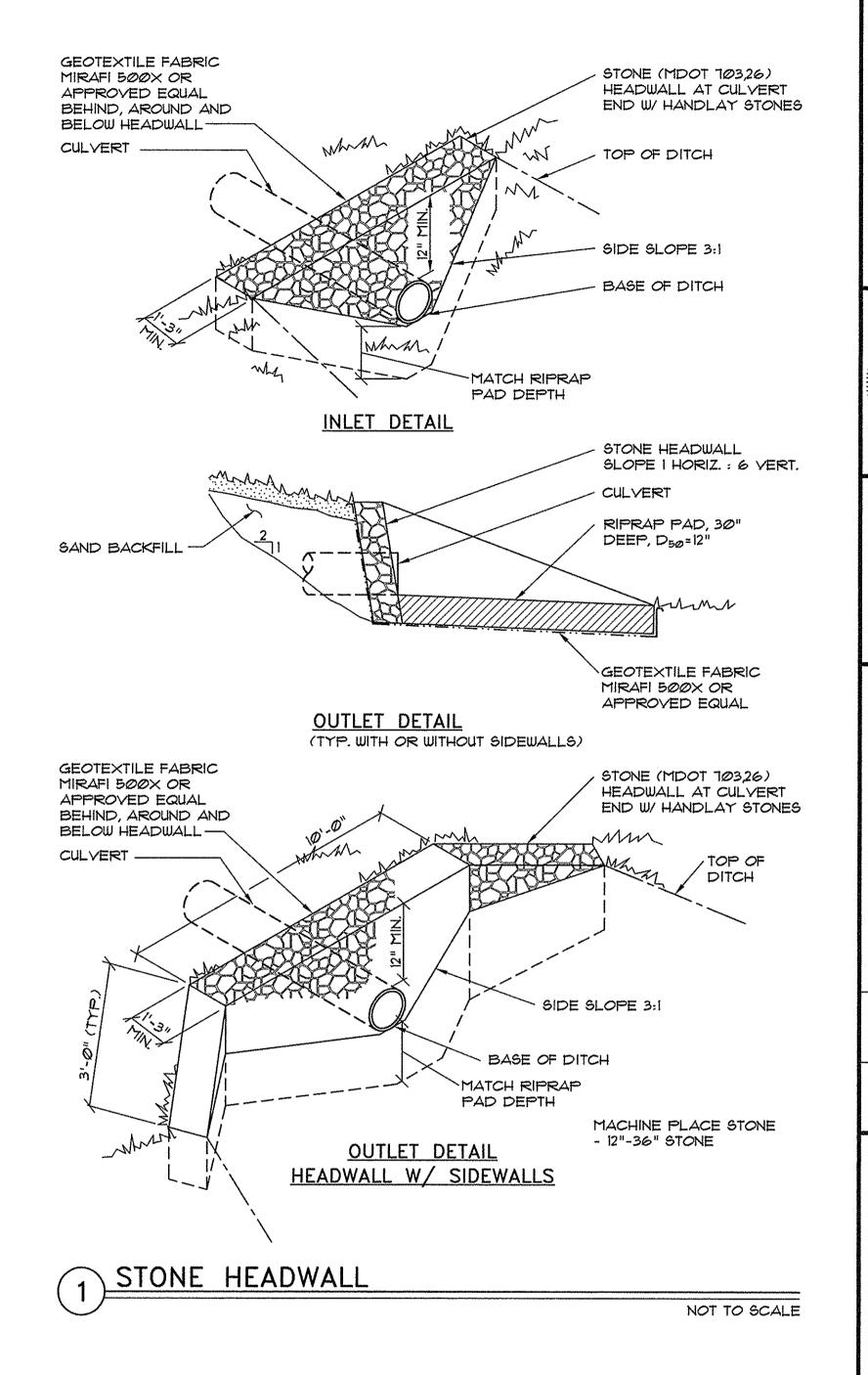
NOTES:

1. INSTALLATION SHALL NOT ALLOW INTER-TWINING OF CABLES.

- DIRECT BURY ELECTRICAL CABLES MAY BE USED IF ALLOWED BY CENTRAL MAINE POWER.
- DIRECT BURY CABLES EXCEPT UNDER PAVED AREAS. PROVIDE SCH. 40 PVC CONDUIT UNDER PAVED AREAS, EXTEND CONDUIT 5'-0" BEYOND EDGE OF PAVEMENT.
- 4. CONFIRM CONDUIT SIZES WITH INDIVIDUAL UTILITY COMPANIES PRIOR TO INTALLATION.
- 5. COORDINATE FINAL LAYOUT WITH INDIVIDUAL UTILITY COMPANIES.

CABLE TRENCH SECTION

NOT TO SCALE



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